

Balanced Erection for New London Bridge

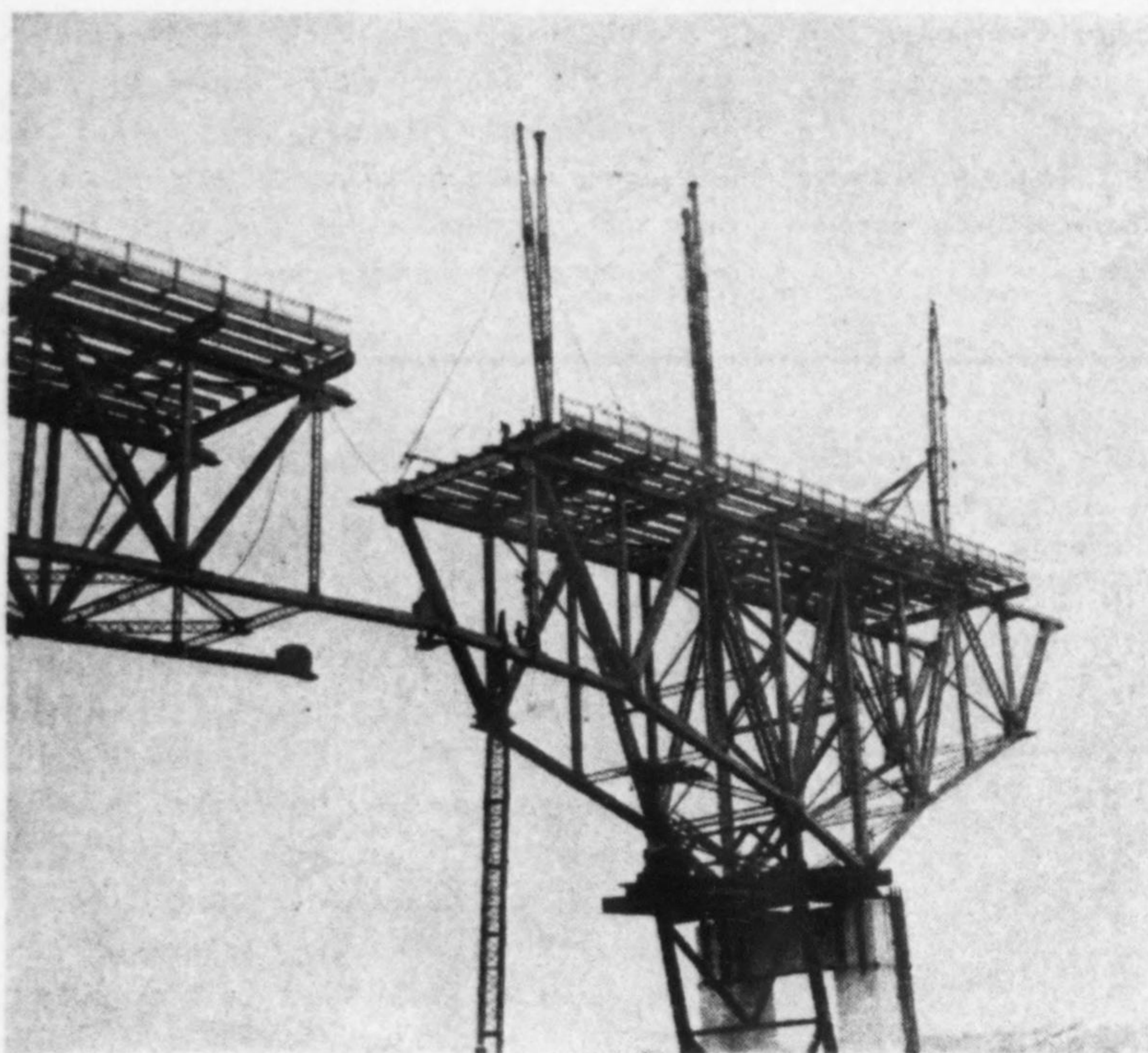
Balanced and cantilever methods were used for erection of the recently completed 5,926-ft. long Groton-New London, Conn. bridge over the Thames River (*ENR*, March 4, 1943, p. 326). The structure provides two sidewalks and two 24-ft. wide roadways separated by a center mall. Main river crossing is a 1,245-ft. deck cantilever with 352.5-ft. anchor arms and a 540-ft. channel span of which 216 ft. is a suspended deck truss. Adjacent to each end of

the main span are four 312.5-ft. deck trusses arranged in two-span continuous units. Deck plate-girders of varying span to conform to existing conditions complete the bridge, which is more than a mile long.

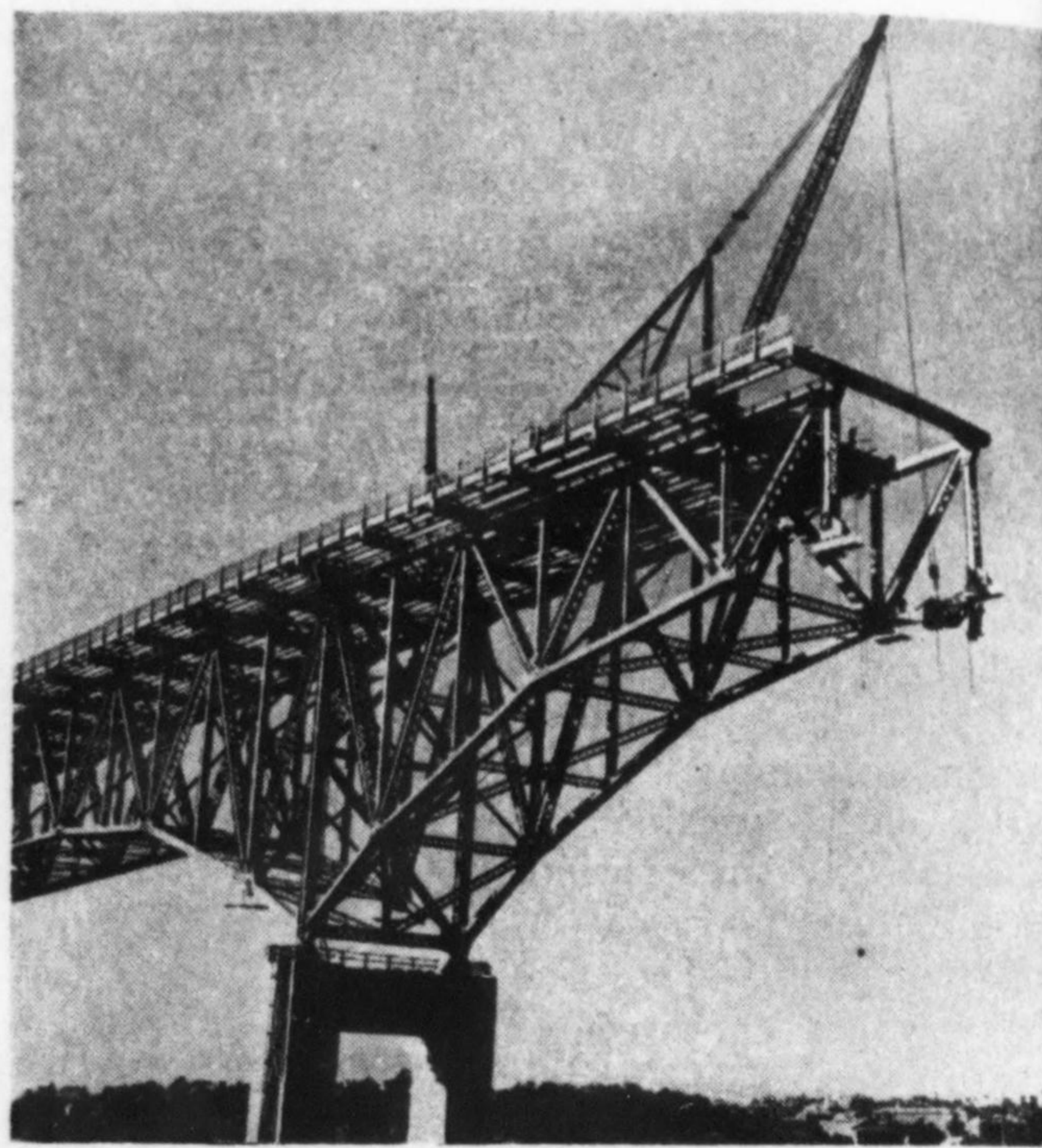
Erection was from each side of the river in three separate operations; erection of approach girders, continuous deck trusses and the main span. The girders were erected by a large crawler crane operating on the ground and raising the

girders in sections. The rest of the trusses were lifted into place by the balanced method and a deck traveler operating on the previously completed spans.

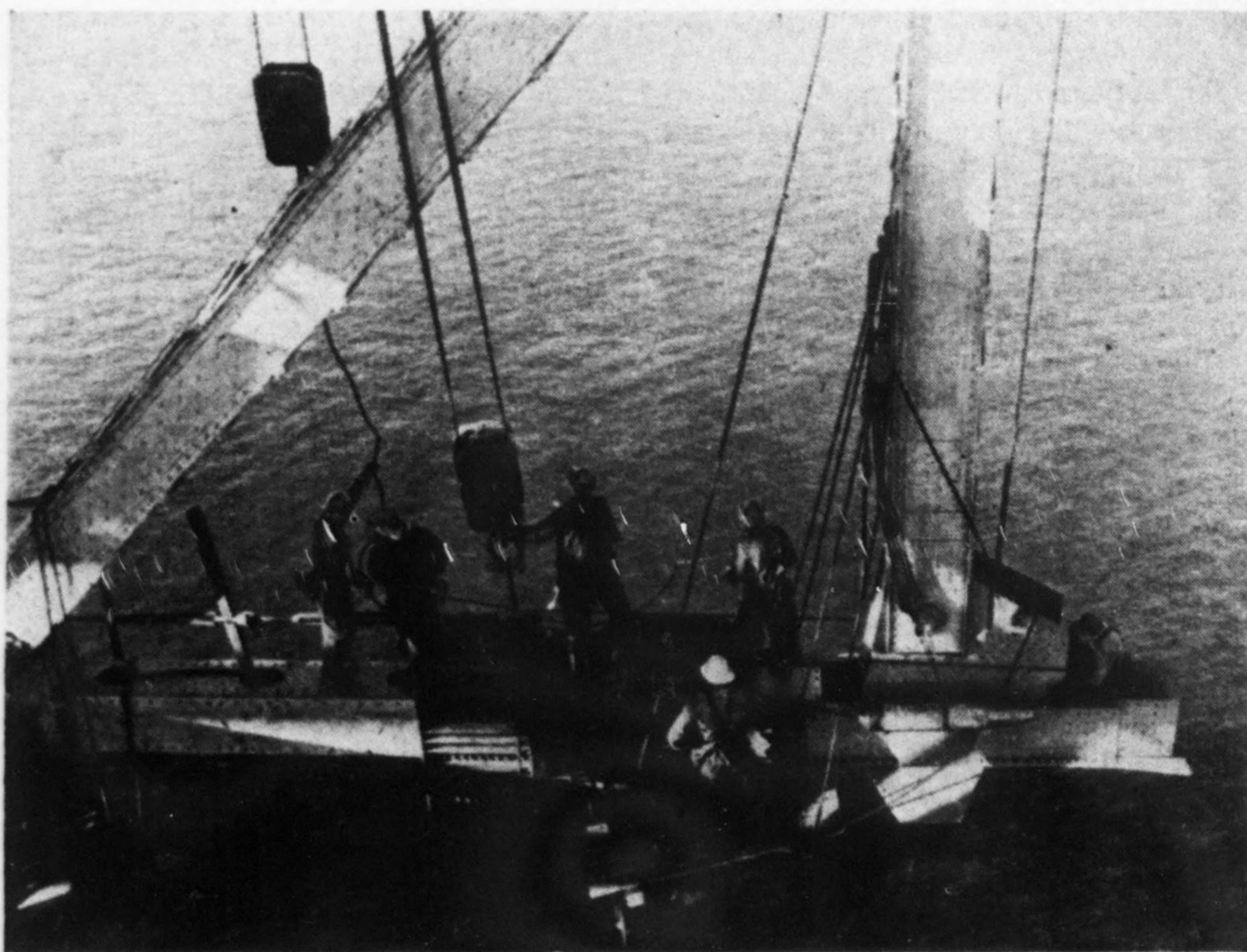
The four 312.5-ft. deck trusses at each end of the main structure, were erected by traveling derricks operating on the deck. Usual procedure was to cantilever out six panels, 186 ft., from one end of a falsework bent, and then to erect the remaining four panels, or 126.5 ft. also by cantilever method.



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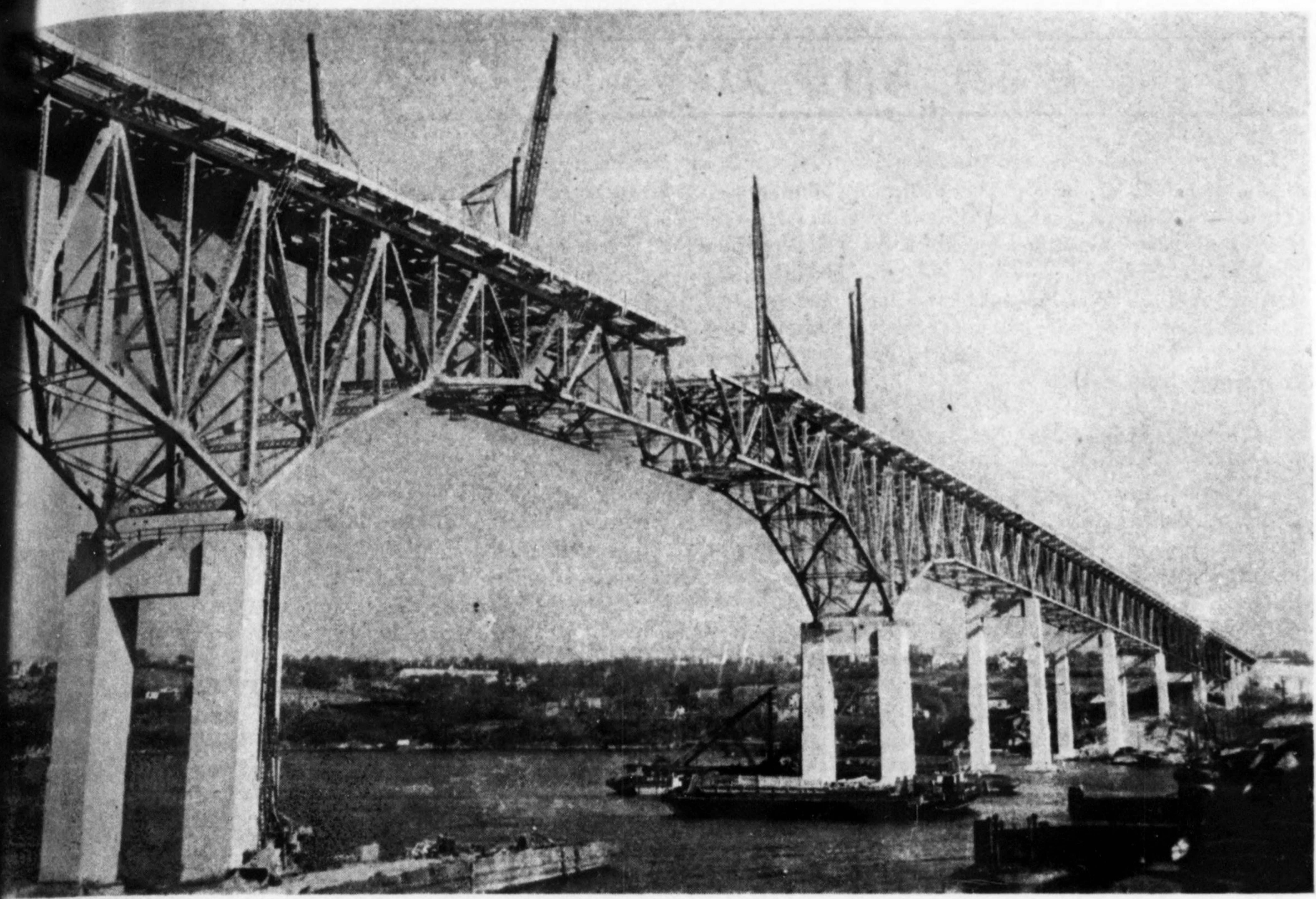
Fig. 1. Balanced erection over a main span to meet the cantilevered section of an arm of the Groton-New London Bridge. Central guy derrick was first set-up on pier. As erection continued the derrick "jumped" to the deck to erect left and right traveler.

Fig. 2. Traveler moves out over cantilevered section of the main channel span. The main fall is used in jacking device for the suspended truss. Similar jacking arrangements were provided for the top chord at either end of the suspended span.

Fig. 3. Close-up of jack installation in Fig. 2. Two 300-ton hydraulic jacks are bolted in the temporary (bolted) steel structure. The main bottom chord of the bridge is visible above and below the main bottom chord. Pin with pilot nut and driving head may be seen just below the panel point.

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4. Suspended span approaching final closure. Right guy derrick was left in position to dismantle the two travelers and the small jinniwink derrick used for placing the diaphragms and miscellaneous small members.

er the pier under the center of the span continuous units no additional metal in the members was necessary to carry the extended cantilevers. However, at the simple support piers a pin in the plane of the top chord and a roller in the plane of the bottom chord were used to permit continuous erection of the steel.

Erection of each 352.5-ft. anchor arm continued by the same cantilever method and with the same equipment in eight 27-ft panels. Over the main span erection was by the balancing and cantilevering shown in Fig. 1, the steel extending to join the anchor arm five 27-ft. panels away from the main pier.

After closure of the balanced section with the shoreward anchor arm, the erection of the 540-ft. channel span was carried on without falsework. The 216-ft. suspended section is pin connected to the top chord. During erection a heavy member, and jacks for closure (Fig. 3) took the compressive stresses of the cantilevered portion of the suspended span. The corresponding top chord member took the tensile stress.

The bridge was built for the Groton-New London Bridge Commission. The design of the structure was done by the Connecticut State Highway Department which W. J. Cox is commissioner and

L. G. Sumner bridge engineer. A. W. Bushell, now deputy commissioner, was in direct charge of the work, with W. G. Brenneke project engineer.

Parsons, Klapp, Brinckerhoff & Douglas were consultants to the Groton-New London Bridge Commission on design and construction and were represented on the work by Irvine P. Gould. Dur-

ing part of the erection Fay, Spofford & Thorndike were consultants to the highway department. For the latter firm R. E. Crawford was resident engineer.

The superstructure was fabricated and erected by Harris Structural Steel Co. of New York. R. Smith was superintendent and E. L. Gerber engineer on erection.

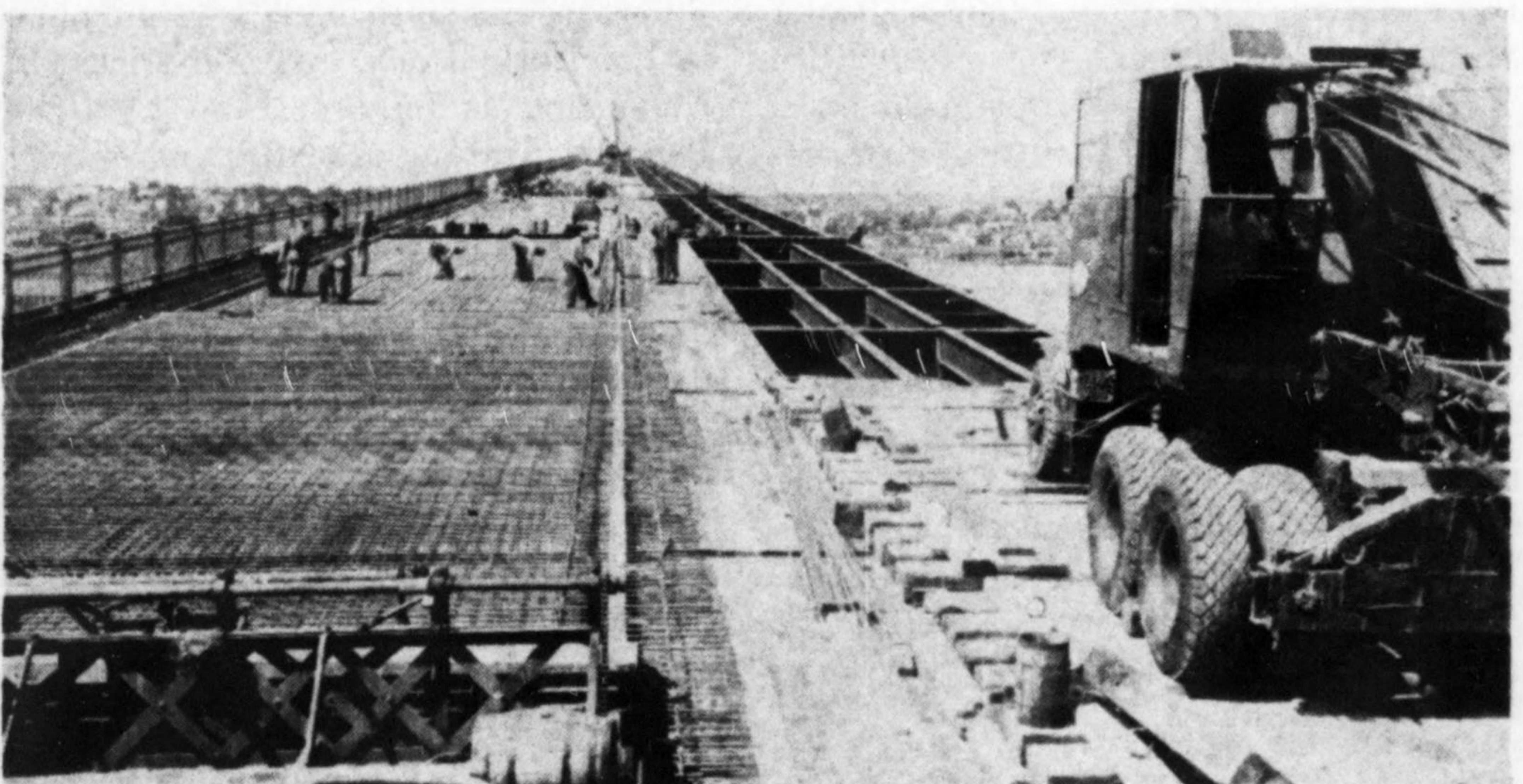


Fig. 5. Forms for the 7 3/4-in. thick concrete deck were carried on kneebraces supported on the lower flanges of the stringers. Approach concrete, supplied by a paver at ground level, was raised and placed by a truck crane. Truck-mounted mixers delivered most of the concrete for the central sections, a crane and bucket being used where trucking was prohibited.