## New cantonment program hinges on passage of extended service bill

With advanced planning already underway and 23 sites selected, an early start on the new program will be possible when funds are made available

With Congressional approval of the plan to keep the Army selectees in service for another 18 months, which is expected to be granted this week, the start of another cantonment construction program to increase housing for additional draftees will no doubt be underway in the very near future. At present one and one-half million men are being housed. This number can be increased by adding to the regular army and by calling up sufficient men under the Selective Service Act to increase those troops from 650,000 to the 900,000 men now authorized. Both of these actions are expected. In addition, it is highly possible that Congress may remove the 900,000 limit and may permit still more troops to be trained under the Selective Service Act. Work to date on the new program includes approval of 23 camp sites and award of architectural-engineering contracts for 11 of the projects. This work

is being done under a special \$15,-000,000 appropriation.

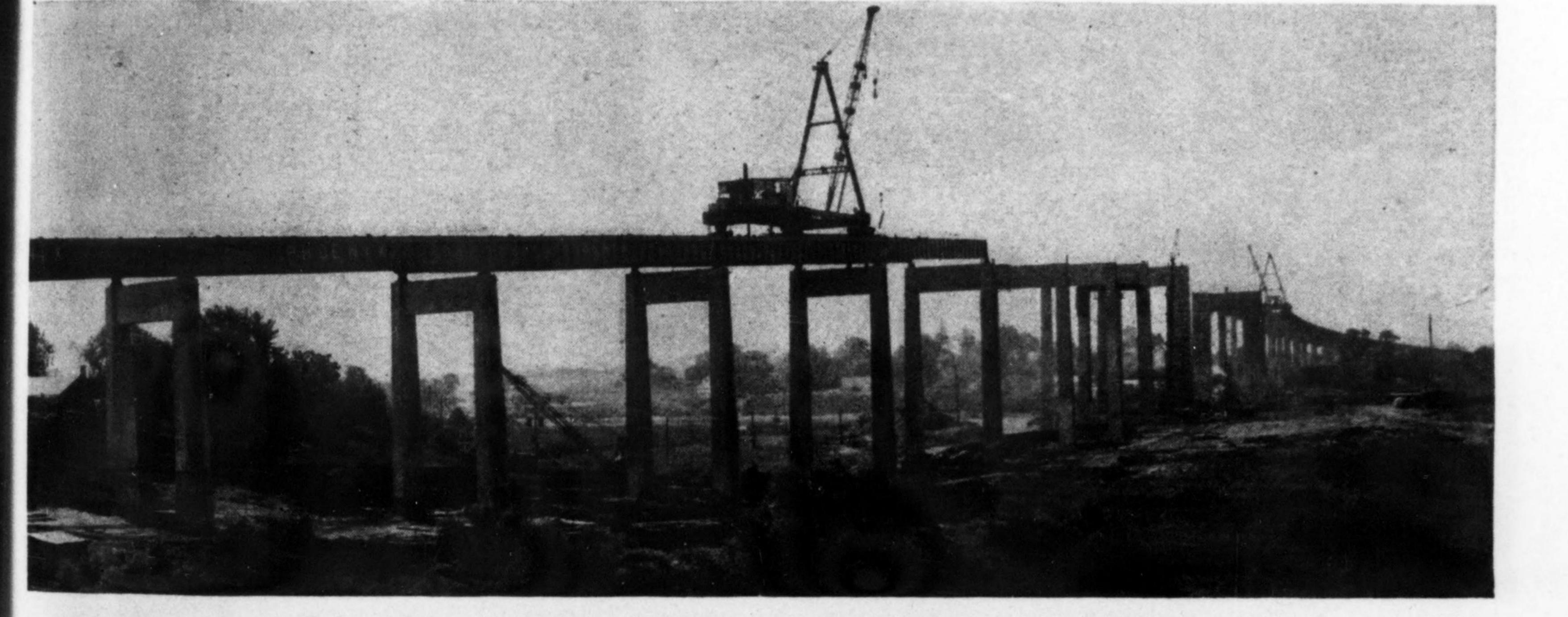
A list of firms holding contracts for the planning work for nine camps was given in ENR May 29, 1941, p. 849. On July 16 the War Department increased the number of firms to eleven by awarding an architectural-engineer contract to Freeze & Nichols, Ft. Worth, Tex., for a camp at Bastrop, Tex., and a similar contract to Fisher, Fisher & Hubbell, Denver, Colo., and Paulette & Wilson, Topeka, Kan., for a cantonment at Colorado Springs, Colo. The remaining twelve sites approved are located as follows: Corvallis, Ore.; Durham, N. C.; Marysville, Calif.; Morganfield, Ky.; Camp McCoy, Wis. (expansion of existing reservation); Ozark-Enterprise, Ala. area; Paris, Texas; West Yellowstone, Mont.; Hopkinsville, Ky.-Clarksville, Tenn area; Waco, Tex.; Fort Huachuca, Ariz.; and Venice, Fla. The average camp is planned to accommodate roughly 30,000 troops, and the average cost is put at \$23,000,000. Specifications call for the standard type construction, similar to that used in last winter's cantonment program. The work may be done by general contract or on a cost-plus-fixed-fee basis. In either case, a much more satisfactory set-up as to payment can be expected because of the advance planning. The firm holding the planning contract for a camp is expected to receive \$50,000 for that work alone and then an additional \$25,000 where supervision of construc-

tion is also handled.

### Structural steel orders double 1940 figures

The American Institute of Steel Construction reports that structural steel orders booked in June were 13 percent ahead of the monthly average for the entire first half of 1941, and new orders received in the first half of this year were more than double those of the same period, in 1940.

The first six months' orders in 1941 totaled 1,298,432 tons compared with 619,231 in the first half of 1940. Shipments for the first six months of this year totaled 1,055,946 tons compared with 655,012 in the first half of 1940.



#### Steel superstructure goes up on St. Georges Bridge

In place of the vertical lift highway bridge at St. Georges, Del., over the Chesapeake and Delaware Canal which was demolished in a ship collision on Jan. 10, 1939, a new highlevel steel tied-arch four-lane bridge is now under construction at a site about 600 ft. west of the original location crossing. All the work on the substructure of the span, contract held by Penker Construction Co., Cincinnati, Ohio, has now been completed.

Rapid progress is being made on the approach superstructure with better than twothirds, of the steelwork completed. Only one span remains to be joined with steel on the south side and only two spans to be joined on the north side.

The main tied-arch span will be 540-ft. long. A connecting link with the DuPont Highway, the arch, giving 135-ft. shipping clearance, is flanked on each end by 1,800 ft. beam and girder approaches. There will be two 23-ft. roadways divided by a 2-ft. curb strip, and one 4-ft. sidewalk. Work was begun on July 1, 1940, and with construction about 70 percent, it is expected that the bridge will be opened to traffic in December. The bridge is being built for the Corps of Engineers with the superstructure contract held by Phoenix Bridge Co. with Harry Archinal, superintendent. Approach contractors are George & Lynch, Wilmington, Del. Robert Fox, was substructure engineer on the job for Penker Construction Co. Parsons, Klapp, Brinckerhoff and Douglas, New York City, are consulting engineers, and William Bruce is acting as resident engineer. H. N. Crichton is resident engineer for the government. Estimated cost, including cost of right of way by State of Delaware is \$2,747,108.43.

ENGINEERING NEWS-RECORD • August 7, 1941

(Vol. p. 187) 3



Fig. 1. A type of tied arch new to America spans the Chesapeake and Delaware Canal.

## Tie Dominates Rib in St. Georges Arch

Maurice N. Quade

Structural Engineer with Parsons, Klapp, Brinckerhoff & Douglas, Engineers

**Contents in Brief**—A bridge type new to American practice is introduced by the new crossing of the Chesapeake and Delaware Canal at St. Georges, Del. It is a 540-ft.-span tied arch, but in contrast to usual practice the tie is the heavy member and the arch rib the light one. Of box section 108 in. deep, the tie resists most of the live load moment and serves as a stiffening girder for the arch rib. Advantages of this type of span appear in simpler details, easier erection, and in a possible small saving in steel.

THE NEW HIGHWAY BRIDGE that carries the DuPont Highway over the Chesapeake and Delaware Canal at St. Georges, Del., fourteen miles south of Wilmington, has a main span consisting of a 540-ft. tied arch of unusual design, its claim to distinction resting in the fact that the bending moments caused by partial live loads are resisted principally by the tension tie rather than the compression rib. Contrary to usual tied arch designs, the tie consists of a deep box girder having approximately thirteen times the moment of inertia of the rib, which is shallow and corresponds quite closely to the proportions of a conventional top chord of a simple truss span of the same length and loading. In its action as a stiffening

member, the tie girder acts for the arch rib in much the same manner as a stiffening girder or truss acts in providing rigidity for a suspension bridge. Although there appears to be a European precedent, it is believed that the St. Georges Bridge introduces a new type of bridge structure to American practice. The bridge, which also includes 3,669 ft. of plate girder spans about equally divided between the two approaches, replaces a lift span that was wrecked by a freight steamer. (ENR, Jan. 12, 1939). But whereas the lift span was in the town of St. Georges, the new bridge is about 600 ft. west and outside the town where right-of-way for the long approaches is less expensive and where

the alignment is better adapted to that of the dual highway on either side of the canal. The four lanes of traffic, which the new bridge provides, remove a bottleneck in this dual highway that was caused by the 24-ft. roadway of the lift bridge. Delays to highway traffic and canal shipping due to the lift bridge are also eliminated by the 540-ft. high level arch span, which furnishes a 135-ft. vertical clearance over a 400-ft. width.

Selection of a single high level span for the canal crossing was based on careful cost considerations, which revealed the following relative capitalized total costs of construction, operation and property acquisition:

Vertical life bridge					0.93
Tied arch or simple span		 			1.00
Cantilever					
Self-anchored suspension			,		1.11
Tunnel					

The desire for uninterrupted traffic flow ruled out the vertical lift, while the tied arch was preferred over the simple truss on the basis of appearance.

The preliminary studies led to the

42 (Vol. p. 22)

January 1, 1942 • ENGINEERING NEWS-RECORD

adoption of a tied-arch main span 539 ft. 41 in. c. to c. of bearings with 15 panels 35 ft.  $11\frac{1}{2}$  in. in length. The arch ribs are straight between panel points, which lie on a parabola having a rise of 100 ft. The tie girders are slightly curved and conform to the vertical curve in the roadway, which is a parabola between tangents to the 5-percent approach grades at the ends of the span. The effective depth at the center is 92.85 ft. A cross-section of the deck is shown in Fig. 3. It will be noted that the stringers are located at approximately the level of the top flange of the deep silicon steel tie girder. In most of the panels, this flange is subject to a wide variation in unit stress due to direct load in combination with live load so placed as to produce maximum negative or maximum positive bending moment in the tie girder. The greatest range of stress is from a minimum of less than 3 kips per sq. in. to the maximum allowable stress of 24 kips per sq. in. in tension on the net section of the member.

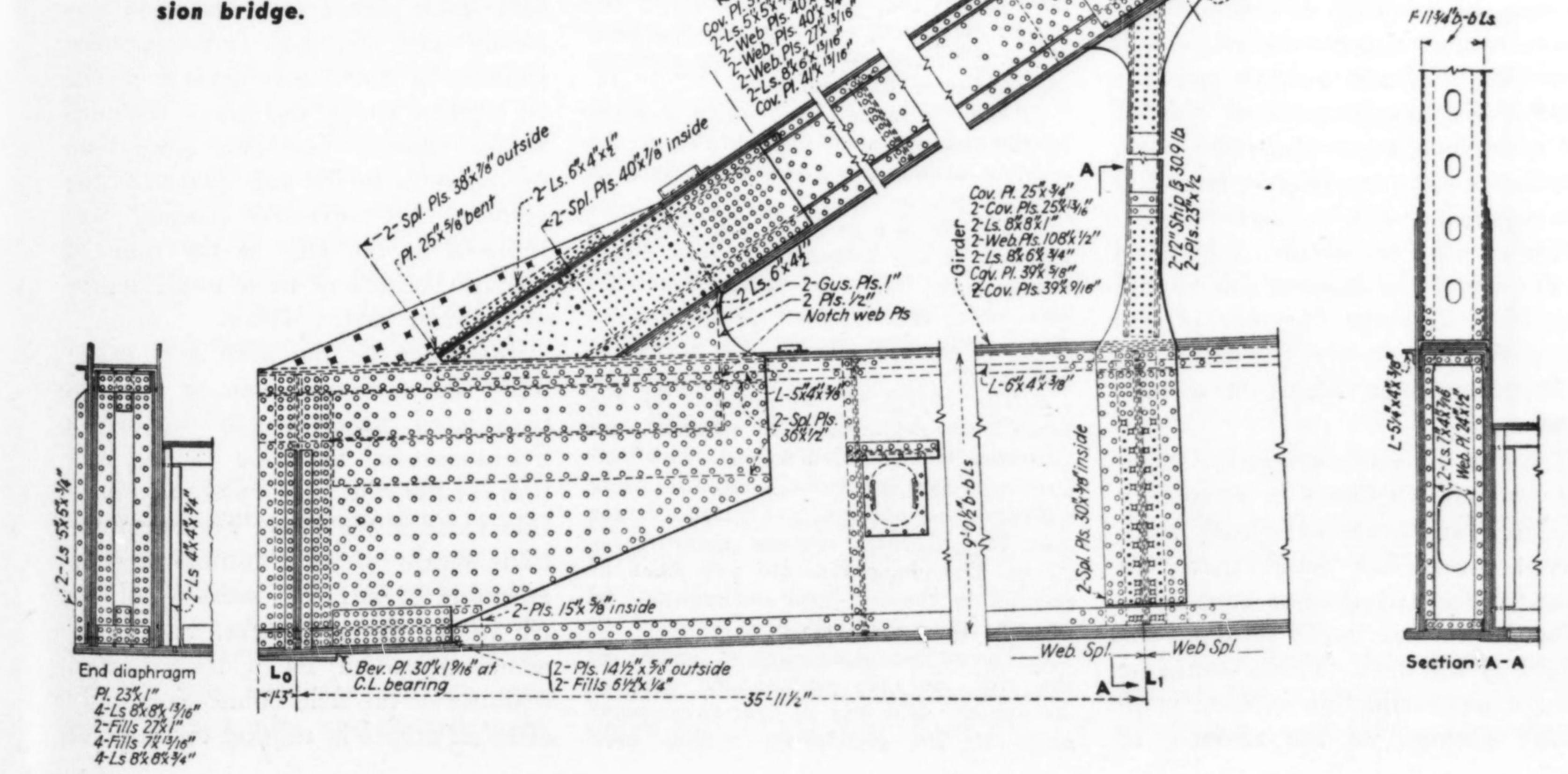
lateral distortion of the floorbeams, expansion joints are used at each intermediate panel point. The slab joint consists of a copper-flashed, poured rubber joint. The stringers on one side of each floorbeam slide on bronze bearing plates having bronze keys to prevent lateral motion. The stringers in the adjacent panel are riveted to the floorbeam flange in order that they may, in combination with the deck slab, brace the compression flange of the floorbeam. A line of rolled channel diaphragms near the ends of both the expansion and fixed stringers prevents rotation of the ends of the stringers. The deck is stiffened by a similar line of diaphragms at mid panel. The arch rib is a box section of plates and angles with web plates 42 in. deep and coverplates top and bottom. Hangers consist of rolled channels having coverplates on both sides. The depths of the channels vary in accordance with the lengths of the hangers to avoid excessive slenderness and flexibility.

selection of a tied arch, is the effective blending of the main span with the girder approach spans. It was found that if the tie girder could be made the same depth as the approach girders, the dominating line of the bridge could be continued across the main span without interruption; that the unity of the construction could be further enhanced by using a relatively thin rib, which would have the appearance of merely aiding the girder to cross the canal in one span. Structurally, the rib functions in exactly that manner. The deep tie girder is so stiff that the rib resists in bending only 7 percent of the bending moment. The tie girder is a double web box section  $108\frac{1}{2}$  in. deep b. to b. of angles. The bottom flange angles are turned out and the top angles turned in, with coverplates on both flanges. Since the web plates are always stressed to at least a small tension, the usual limiting thickness for ordinary plate girder webs did not need to be considered. The largest members are located near the quarterpoints and have a gross area of 299 sq. in. and

To avoid large participation stresses in the floor system and accumulative

An important esthetic consideration, which in part determined the

Fig. 2. Details of the tie and rib connection on the St. Georges arch. The tie is thirteen times as stiff as the rib, and the interaction of the two members is comparable to that between the stiffening truss and the cable of a suspension bridge.



ENGINEERING NEWS-RECORD • January 1, 1942

(Vol. p. 23) 43

a net section modulus of 9,640 in."

The lateral bracing for the rib is a K system. At each portal the end transverse strut is omitted and, to insure proper stiffness and equal division of transverse shear in the end panels of the rib, a longitudinal strut at the center line was added to each K panel adjacent to the portals. The bottom lateral bracing is placed at the bottom of the tie girders and is also a K system. Details and sizes of members are given on Fig. 3. Access to the truss members is provided by manholes in the rib and tie girder and handholes in the hangers. The manholes in the rib are in the bottom coverplate and are spaced on 5-ft. centers. One manhole having a removable cover is provided near the center of each panel of the tie girder, at the center of the inside web plate to avoid large holes in the heavy flanges. Additional manholes are proprovided in the diaphragms at the panel points and at mid-panel so that once the tie girder is entered the entire length of the girder is accessible. The girders can also be entered at the ends of the span from walkway extensions to the platforms below the deck, which are provided for splicing telephone and power cables carried on the bridge. The intermediate manholes were added for convenience in painting and for ventilation. Special shop paint was used on the inside of the girders to eliminate poisonous fumes from paint burned during field riveting. At the north end of the bridge the span is supported by cast steel fixed shoes of conventional design. Expansion shoes with a nest of segmental rollers support the south end. Provision is made at this pier for jacking diaphragms on each side of the main diaphragm above the shoe. The entire span can thus be raised at the expansion end to make adjustments or even replacements of parts of the expansion shoes if this should ever be necessary. Because of the heavy tie girders, the provision for jacking has been added at a very small cost.

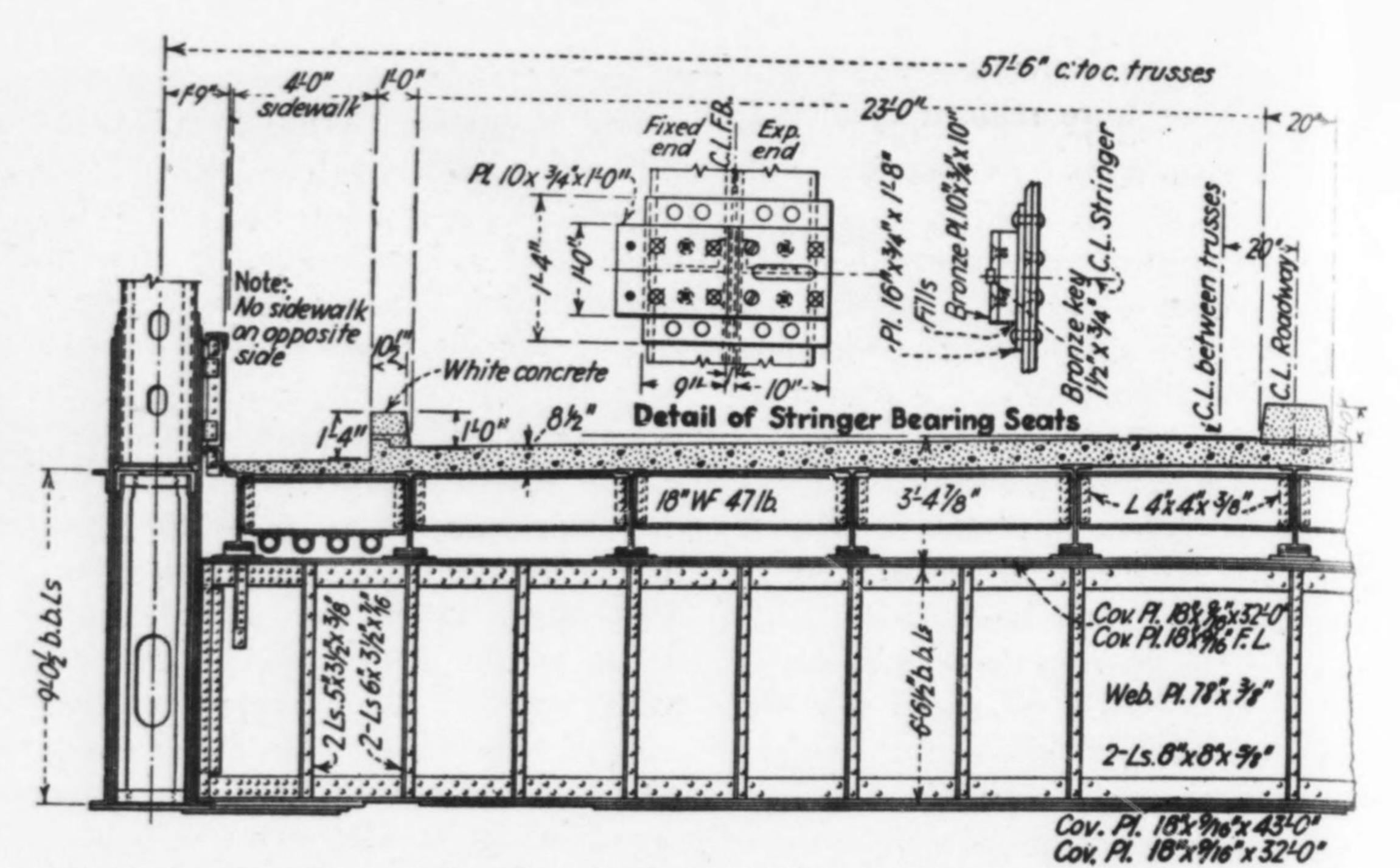


Fig. 3. Cross-section of deck of St. Georges tied arch. The solid concrete deck contributes desirable stiffness. One end of each stringer is on an expansion bearing to avoid floorbeam distortion.

bending moments resulting from uniform load throughout the length of the span. At the same cost, the heavier dead load is preferable because of greater stiffness.

#### Design of the arch span

The arch truss is statically determinate for external reaction but is highly indeterminate internally. In the first analysis certain assumptions completed, and the variation in sections was taken into account.

Due to rib shortening and the lengthening of the tie girder under stress, there is a moment in each member when the structure is loaded, even with uniform load throughout its entire length. To correct this approximately, a moment of opposite sign can be applied to the ribs and tie girders at the time of closing the span. A pin was therefore placed in each tie girder at the center of the span far enough above the neutral axis to induce a bending moment of 500 kip-ft. at the center of the span. Since the span was closed with little more than the weight of steel structure upon it, this moment included an allowance for the effect of the concrete deck placed after the span was swung and riveted, including the riveting of the flange splices in the tie girders above and below the pins at the center. The pins were connected only to the web plates of the girders. No corrective moment was induced in the ribs at the time of closure because of their small degree of moment participation. Inasmuch as the arch acts externally like a simple span, it was not considered necessary to employ a field-measured keystone closing section for the ribs, which were carefully fabricated in the shop. The specifications required, as a minimum, shop assembly of adjacent sections of the rib and of the tie girders, with alignment controlled by a transit, and reaming of the field connections only after all errors in milling the ends of

were made to simplify the problem and, as the members approached their final sections as a result of successive analyses, the effects of those assumptions that might have an appreciable influence on the design were investigated. For the preliminary design it was assumed that:

1. The hangers, being long and slender, do not resist enough of the bending moment to have an appreciable effect on the design of the rib and tie. A comparison of the l/l ratios of the members confirms the validity of this assumption.

2. The rib carries no bending moment. In the subsequent and final designs the bending moment was proportioned between the rib and the tie.

3. The rib is a parabola that remains a parabola in the deflected structure. Since the truss was fully cambered for dead load and since the live load deflections are small, little error is introduced by this assumption.

The ribs, tie girders, and the short, end hangers (because of secondary bending) are made of silicon steel. Except for a few detail parts, all other members are carbon steel.

No effort was made to limit the weight of the deck. There would be little, if any, reduction in cost, principally because of the absence of

4. The effects of deformations of the hangers are negligible. The hangers were shortened for dead load tension. Live load stresses are small because, to obtain proper stiffness, the hangers were made larger than the minimum sections required for stress. The elongations are very small in relation to the live load deformations in the ribs and tie girders.

5. The area and moment of inertia of the ribs and tie girders are uniform throughout. This was, of course, corrected after the first preliminary designs were

44 (Vol. p. 24)

#### January 1, 1942 • ENGINEERING NEWS-RECORD

the ribs were corrected. The hanger connections were reamed to a metal template.

#### Approach spans

At the lower end of each approach there are several stringer spans 54 ft. long. All other approach spans are deck plate girders varying from 80 to 130 ft. in length. The girders in the 130-ft. spans are silicon steel; all other members are carbon steel.

All spans are simple except for a

piers are founded on steel H-piles. The piers supporting the truss and girder spans are of open construction and have twin shafts connected at the top with a wide, deep strut, the dimensions of which vary in accordance with the height of the pier. The struts at the main piers are  $8\frac{1}{2}$  ft. wide and 15 ft. deep. The clear width between shafts is 37 ft.

The depth of the main piers has been fixed at a depth of 50 ft. below mean low water. This is the highest elevation that can be considered satisfactory for the maximum channel that could ultimately be constructed under the main span. The steel sheetpiling of the cofferdam, within which each pier was constructed, was driven 10 ft. below the base, the sheets were cut off at the top of the base, and the lower portions left in place. Similar protection was provided for the adjacent approach piers. Thus, at a moderate increase in cost, the piers have been adequately pro- concrete subjected to combined bendtected against dredging a possible future canal prism 42 ft. deep and 500 ft. wide on the bottom.

The tallest of the approach pier shafts is 125.5 ft. above the top of the base. Wherever this height exceeded 75 ft. a transverse strut was added at the bottom of the shafts. which is generally below ground. This strut has been added because the pier bases would otherwise be excessively wide due to large moments from transverse loads. The struts have been proportioned with regard to the interaction between the bottom strut and the bases, so that when the

two-span continuous crossing of a road intersection on the north side. The center pier of the continuous spans is skewed, and the spans are unequal in length. Expansion joints are provided at alternate piers, where the girder spans rest on cast steel rocker shoes. Expansion bearings at the stringer spans consist of bronze bearing plates on which the stringers slide.

The concrete deck is identical with that on the main span. The girders are spaced 42 ft. c. to c. with the sidewalk and curb cantilevered beyond the girders on the east side and a 5-ft. width of the roadway plus the curb cantilevered on the west side. All of the fascia stringers and brackets are 24-ft. wide-flange beams and thus present a neat, uniform appearance. Floorbeams on the girder spans are plate girders 5 ft. deep except at the ends of the spans where the end floorbeams are close together. Here one floorbeam is 5 ft. deep while the other, on the adjacent span, is 4 ft. deep to offset the bottom flanges vertically and provide easier access between floorbeams for painting. On each approach, stairs are provided for pedestrian access between the bridge deck and the street nearest the canal. The stairs are approximately 50 ft. high. The upper flights above the pier tops are steel. The lower flights wind around one of the pier shafts and consist of a continuous spiral concrete cantilever slab the upper surface of which conforms to the arrangement of steps and plat-

At the north pier both the main span and the 130-ft. approach span are fixed, but at the south pier both spans rest on expansion shoes, a cast steel, toothed, cantilever type of expansion joint being provided for the deck. Because of the overturning effect of a longitudinal load of 346 kips (estimated as 560 and 350 lb. per ft. of bridge for the main span and approach span respectively) applied horizontally through the fixed shoe pins, two additional transverse rows of steel H-piles were required at the north pier. A similar distinction between fixed and expansion piers was made in the design of the approach piers. The outer rows of piles in the main pier bases are battered outward for increased stability, particularly during future dredging when unbalanced earth pressures may occur temporarily.

strut is fully stressed to allowable unit stresses the maximum deflection of the outer rows of piles due to rotation of the ends of the strut does not exceed is in., which is well within the limit of elastic action of the pile.

In piers that have no bottom strut, points of contraflexure were assumed at about the lower third point of the shafts on the basis of a detailed study of a typical pier.

The maximum unit stress used for ing and direct stress, with wind included, is 700 psi. This intensity occurs at the tops of the shafts where transverse wind moments are large. At the bottom the shafts are designed for 450 psi. or less, with the additional requirement that the bending stress from all sources shall not exceed the direct stress by more than 100 psi. when expressed in terms of a homogeneous material. Sections that are critical for diagonal tension are proportioned for a shear of 50 psi. or less to avoid web reinforcement. The maximum unit stresses in the reinforcing steel are 18,000 psi. for dead plus live load, or for dead plus wind load, and 22,500 psi. for combined dead plus live plus wind loads. At the suggestion of the architect, the tops of the main piers have been completely enclosed up to level of the deck with corrugated asbestos-cement board, which is attached to light steel framing anchored to the pier top. Two ornamental pylons are placed on the ends of each pier enclosure outside of the railing at the corners of the main span. The pylons are starshaped in section with a slender finial at the top, which is 22 ft. above the roadway. They are made of aluminum and glass, and are illuminated by light strips 12 ft. high.

In resisting longitudinal loads, the piers act as vertical cantilevers and are reinforced accordingly. In resisting transverse loads, the horizontal struts act with the shafts as a framed bent, and the pier as a whole has been the two main piers has a single base; girders have separated bases, one

forms.

#### Substructure

The substructure consists of 41 piers and 2 abutments, all of which reinforced in accordance with the are founded on piles. Along the bending moments determined at all higher ground toward the ends of the points in the frame. Each shaft of approaches, cast-in-place concrete piles are used. Nearer the canal, the piers supporting the approach groundwater near the surface permitted timber piles, and the two main under each of the twin shafts.

#### Advantages of this type of arch

It is believed that this type of through tied arch has the following advantages over the conventional

ENGINEERING NEWS-RECORD • January 1, 1942

(Vol. p. 25) 45

heavy rib type, in addition to its superior appearance in many locations:

1. The erection will generally require less falsework because the tie girders will support erection equipment over longer spans.

2. The demands on erection equipment are reduced by reason of the fact that the heaviest members are lifted only to the level of the deck rather than to the top of the span. This may in some designs eliminate the necessity of special creeper travelers moving along the rib and permit the use of more common equipment operating from the deck.

3. Although no direct comparison in weight has been made as yet, it can be pointed out that some economy may result from the shorter length of the heavy tie as compared to the longer length of a heavy rib. This saving is partly offset by heavier splices.

4. Since the tie girder will always have little, if any, compression, the ends of the members at splice points need not be milled. All milling of the splices in the curved rib is therefore confined to lighter, smaller members.

span appear to be simplified when the office at Chesapeake City, Md.

deep girders are horizontal or nearly so, and the transfer of the vertical shear into the shoes is readily accomplished by detail parts that are easier to fabricate. The erection in this panel is much less difficult because a milled splice for the first rib member can generally be located above the tie girder so that the first section of the rib can be erected after the girder has been placed on the shoe and the first hanger erected. No temporary support for a supervised the field work, with W. H. heavy, inclined section of rib is required.

The bridge was built by the United States government through the agency of the Corps of Engineers, U. S. Army, with Lt. Col. H. B. Vaughan, Penker Construction Co., Cincinnati, District Engineer of the U. S. Engi- Ohio, and the superstructure by the neer Office at Philadelphia, as the Phoenix Bridge Co., Phoenixville, contracting officer. Preliminary work Pennsylvania, with the Corbetta Conon the project was carried out under struction Co., Inc., of New York, as the direction of Lt. Col. C. W. Burlin, subcontractors for the concrete deck District Engineer until January, 1940. and J. R. Umberger Co., Lebanon, General supervision of the field work Pa., as subcontractor for the elecfor the Corps was under H. N. Crich- trical work. The approach roadways 5. The connections at the ends of the ton, principal engineer of the sub- were built by George and Lynch, Wil-



William H. Bruce Charles U. Kring and **Resident Engineer** Assistant Resident Engineer Parsons, Klapp, Brinckeroff & Douglas, Engineers, New York, N. Y.

Contents in Brief—The heavy tie girders of the St. Georges steel arch simplified the erection problem. Different cantilever erection schemes, one involving a tie-back and the other temporary diagonal web members, were used for the two ends of the span, after which the center portion of tie girder served as a suspended span on which hangers and arch ribs were erected.

THE ERECTION of the 540-ft. tied arch a structure conforming to the correct accomplished by an unusual combina- under full dead load. tion of conventional methods of erecthe span.

second problem was to obtain finally hoisted from barges in the canal.

46 (Vol. p. 26)

An A-frame traveler, weighing 150 top chords to the erection bent at L4. tion. The War Department prohibited tons, which had previously erected the In order to cantilever beyond the

#### Personnel

Plans and specifications for the complete project and a preliminar report in 1939, were prepared b Parsons, Klapp, Brinckerhoff & Doug. las, Engineers, New York, under the supervision of E. L. Macdonald partner, the writer being in charge of the engineering work. This firm also Bruce as its resident engineer. Shortridge Hardesty was associated in the engineering design, and Aymar Enbury II served as architectural consultant.

The substructure was built by the mington, Del.

## How St. Georges Tied Arch Was Erected

On the north side, falsework bents placed at the second and fourth panel points permitted conventional cantilever erection beyond the fourth panel to midspan, although temporary diagonals were added to the web system of the trusses and, where necessary, hangers were temporarily reinforced of the St. Georges Bridge over the geometrical outline and free of sec- for compression stresses. A tempo-Chesapeake and Delaware Canal was ondary moments in the truss members rary sway frame was also added at U<sub>4</sub> L<sub>4</sub> to transfer lateral loads on the

the use of falsework within the present 1,800-ft. north approach, erected the one bent permitted on the south side, 250-ft. width of channel, and since north half of the main span to mid- a tie-back anchored to the adjacent the two piers were at different dis- span. A similar traveler, which had approach span was used. It consisted tances from the channel (Fig. 2) it erected the south approach, erected of a 100-ft. tower above the falsework was necessary to use a different erec- the south half of the main span to bent and tension diagonals from the tion procedure for the two halves of panel point L4S. It was then replaced tower to panel points L4S and to Lo. by a smaller traveler weighing 75 The latter connection was necessary One of the two major erection prob- tons. The steel was hoisted from to prevent a possible reversal of stress lems was to determine the interaction trucks near the two main piers and in the forward diagonals during the of the two halves so that the amount carried out in the falls or on dolly earlier stages of erection because, of temporary erection steel could be cars, except the members in the five from the time the tie-back system was economically yet safely designed. The panels over the channel, which were in place until the span was swung, the south end of the span was not allowed

January 1, 1942 • ENGINEERING NEWS-RECORD

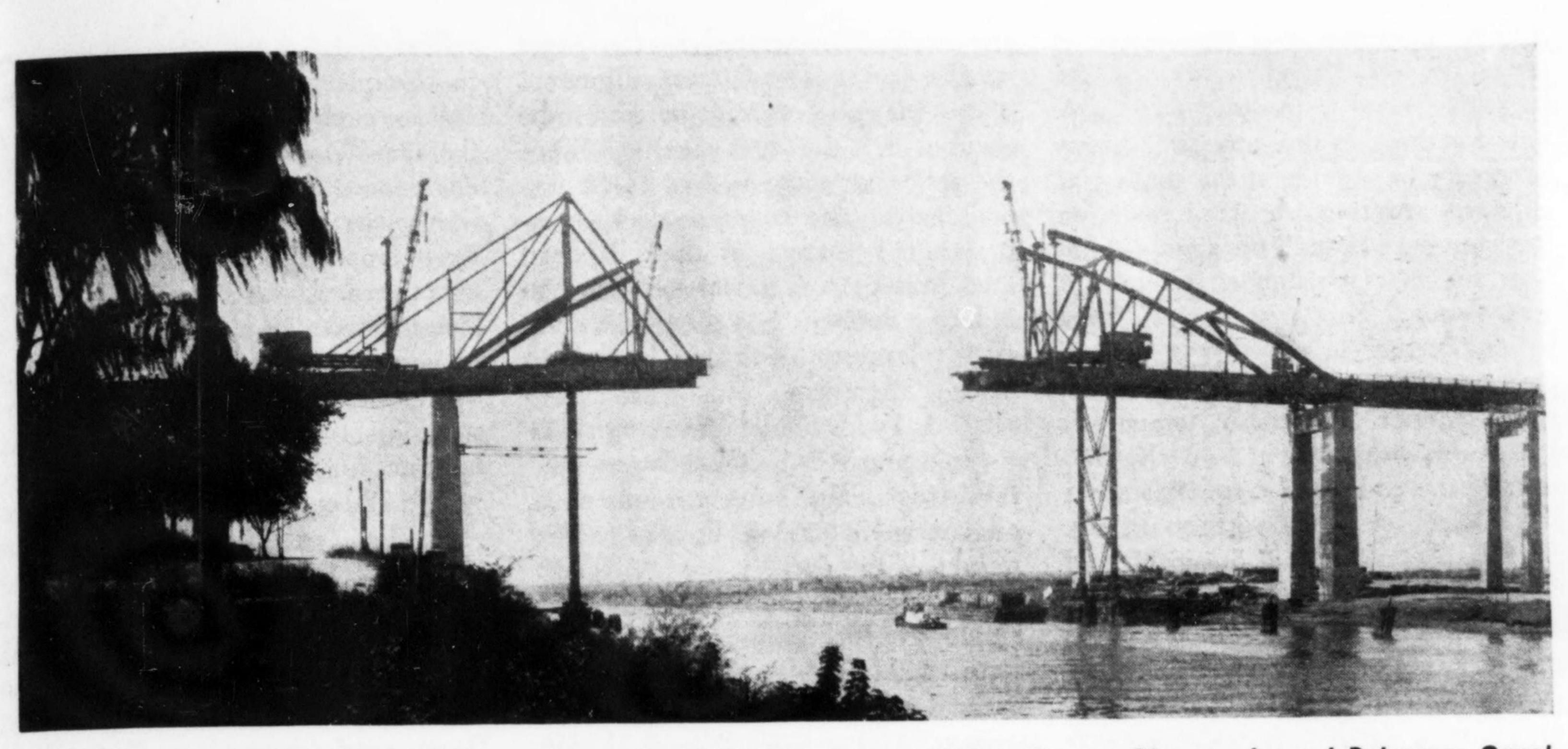


Fig. 1. Two variations of cantilever erection methods being used on tied arch over Chesapeake and Delaware Canal.

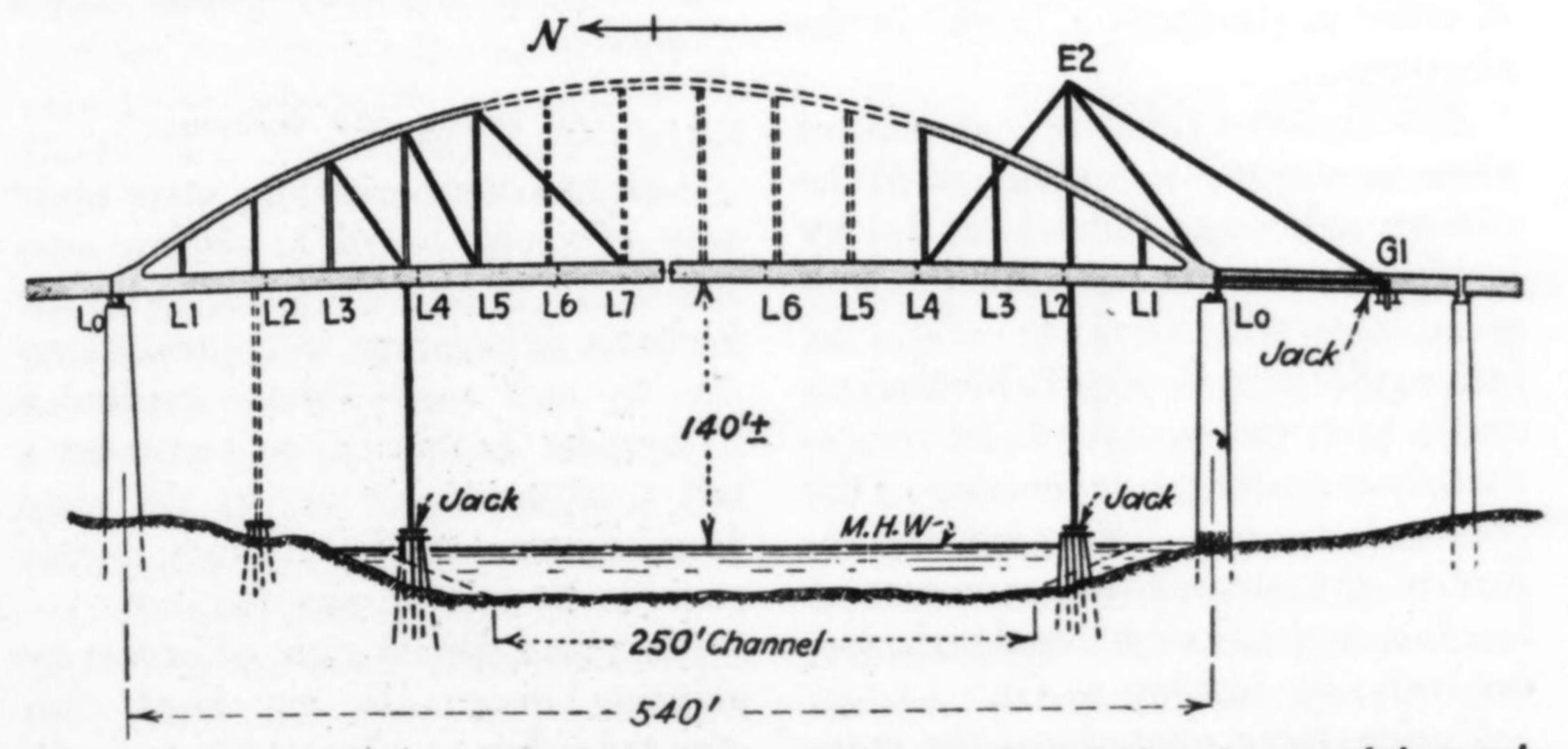
to rest on the pier. L28 and G1 (Fig. 2) were the only points of support.

If the entire south half had been ribs were erected.

tions of the midspan ends of the two main piers. halves were computed for the various elevation was equal then to the load transferred. The north half was statically indeterminate to the third degree and was relatively stiff. The ends of the tie-girders deflected only  $1\frac{1}{2}$  in. with all steel in place. The south half, of first order indeterminacy, was extremely flexible because of the length of the cantilevered girders, and the ends deflected about  $28\frac{1}{2}$  in. at midspan. The maximum load transferred through the pins was approximately 340 kips and occurred just previous to the swinging of the span. Throughout the erection, the load transferred was regulated and checked by use of the hydraulic jacks under the two bents and at G<sub>1</sub>.

Although both the tie-back system north half, so determined that the

The south half was, therefore, adclosure. After the tie-girders were comstages of erection. The force neces- justed by the hydraulic jacks to a sary to bring the two ends to the same computed elevation in relation to the pletely erected it was necessary to



ENGINEERING NEWS-RECORD • January 1, 1942

and the girders of the south half were erection of the closing sections would designed to permit cantilevering the not increase the moment at Las. After cantilevered beyond L28 to midspan, tie-girders to midspan, this procedure erecting the girders to L68, the first a very heavy and extensive tie-back was not followed in erecting the clos- closing section was held in the falls system would have been required. It ing sections of the tie-girders, so as while the splice at Los was fitted up was desirable, therefore, to transfer a not to disturb the alignment of an un- completely. Next the section was portion of the load to the north side. riveted girder splice at L48 when the hoisted sufficiently to bring the pin Consequently, the tie-back system was closing section was added. Had that holes in its north end to the same eledesigned for cantilevering only the splice been riveted immediately after vation as those in the section to which tie-girders and bottom lateral system erection, as originally planned, and it connected, and then held there on beyond the fourth panel point. After while the member was still supported the edges of the web splice plates the tie-girders were connected at mid- in the falls, the alignment of this while the closing section of the other span by a temporary pin, they func- splice would have been maintained. girder was erected. Finally, the two tioned, between L4s and the pin, as a However, at this stage of the erection halves of the span, which had been "suspended" span, partially fixed at the south bent could have been seri- kept approximately 6 in. apart horione end and simply supported at the ously overloaded by a heavy wind. zontally to admit the closing sections other, on which the hangers and arch Consequently, it was considered pref- of the tie-girders, were brought toerable to close the tie girders and gether and the pins driven. Since the In order to find the load trans- bottom lateral system without delay north half was fixed at the main pier, ferred through the pins, the deflec- and thus transfer the wind load to the it was necessary to move the south half by means of jacks for this

> Fig. 2. Erection scheme for St. Georges Bridge. Restriction to one falsework bent on south side made use of a temporary tie-back system necessary. Heavy tie member of arch acts as girder span between cantilever points.

> > (Vol. p. 27) 47

permit expansion and contraction at and at G1 as measured by the gages milled ends of the rib sections were the shoes during the stages when a The effective results of the methods the direction of the Corps of Engismall amount of uplift occurred at the employed for aligning the girder neers, U.S. Army, with Lt. Col. H. B. main pier. The uplift was resisted by were established by a check on the Vaughan, district engineer at Philalong anchor bolts through the shoes, vertical alignment after the span was delphia, as the contracting officer. which were further extended tempo- swung. At no point does it deviate Design and supervision of construcrarily by sleeve nuts and rods to en- appreciably from a smooth curve. tion was carried out by Parsons, gage the trusses.

except those at midspan, were aligned and riveted, the smaller traveler comers and top laterals. The span, which had been held about 12 in. high at the center for clearance in erecting the closing rib sections, was then swung by simultaneously slacking off on all the jacks.

One of the important considerations throughout the erection was the attainment of the proper vertical alignment of the tie-girder, which, being some 13 times as stiff as the arch rib, would determine the geometrical outline of the whole structure. Before riveting, therefore, the moment in every splice was reduced to approximately zero, and the alignment across the splice checked with levels taken on the girder at the panel points. In some instances it was first necessary to subject the splice to a reverse moment, with bolts loosened, in order to eliminate a "kink" in the alignment.

Zero moment (and reverse moment when necessary) was obtained in the sure of the girders, the moments at payable by tolls to pay for cost. cessively, by jacking at G<sub>1</sub>.

puted movements at G1 were checked way, and gave the State Roads Comagainst the reactions at both bents mission power to designate points of

48 (Vol. p. 28)

the south end; the anchorages of the on the jacks. The correct alignment in complete contact. Most of the tie-back system were designed to pro- across the splice at midspan had to be splices came into full bearing when vide for this. At the same time it was secured in a different manner. Since the span was swung, and the remainnecessary to provide at the south pier the splice at midspan had to be riv- der came together very satisfactorily a lateral reaction for wind loads on eted before the concrete deck, com- when part of the bolts in the splice the span even though the span did not prising 43 percent of the full dead were loosened. The placing of the bear on its expansion shoes until it load, was placed, it was necessary to concrete deck was not started until all was swung. This was accomplished induce a moment into the girder suffi- riveting was completed. The sequence by embedding in the pier a vertical ciently large to bring that splice into of construction of the concrete deck steel cantilever beam engaging, in the proper alignment. This was accom- was controlled in order to avoid oversame manner as a tooth, temporary plished by locating the temporary stressing the tie-girders by unsymtransverse bearing seats on the bot- closing pin 9 in. above the axis of metrical loading. tom of the end floorbeam. A similar the girder. The induced moment is, The Phoenix Bridge Co., Phoenixbeam was used at the north main pier of course, removed by the added ville, Pa., was contractor for the because of the lateral instability of weight of the deck. superstructure. All work was under

After all splices in the tie-girders, the ribs was not carried out until the Engineers.

## pleted the erection of the ribs, hang- Many Changes of Engineering Interest In Highway Legislation of 1941

LAWS RELATING TO HIGHWAYS were access. It also authorized a toll superput on statute books of 40 states dur- highway between Baltimore and ing the early months of 1941. Many Washington. related to routine matters, but in an unusually large number of instances roads; this covers the \$50,000,000 Denew trends such as the growing ac- troit crosstown project and proposed ceptance of limited-access roads were routes between Detroit, Toledo and evident. The following brief summary, Grand Rapids. New York created the covering subjects relating particularly Westchester Cross-County Parkway to engineering aspects of highway Authority, with power to charge tolls. development, has been compiled from Ohio authorized the director of highdigests of the new legislation pub- ways to construct freeways or limitedlished in the July number of "Ameri- access roads and to study the locacan Highways," official organ of the tion of a turnpike across the state as American Association of State High- an extension of the Pennsylvania turnway Officials, and in the July 22 bulle- pike. Vermont transferred the admintin of the National Highway Users istration of the Missisquoi Bay Toll Conference.

#### Toll roads and freeways

Colorado authorized its state highvarious splices as follows: at L2N by way advisory board to declare cerjacking the bent at LAN while the bent tain roads to be "freeways," with the West Virginia) voted to submit to at L<sub>2N</sub> was still in place; at L<sub>2S</sub> by highway department in control of ac- public vote amendments to their conjacking down at G1 with Lo blocked up cess to such roads. Maine created a stitutions prohibiting the diversion of on the pier and member E2-Lo discon- "Turnpike Authority" to construct a highway funds. Such amendments nected; at LAN by the connection of the toll road northeast across the state have been adopted in 12 states. On diagonals U4N-L5N. Following the clo- from Kittery to Fort Kent; bonds the other hand, Colorado created a

abutting owners to any road con- ment revenue. During these operations, the com- structed either as a freeway or a park-

Riveting of the bearing splices in Klapp, Brinckerhoff & Douglas,

Michigan authorized limited-access Bridge from a special commission to the state highway board.

#### Diversion of highway funds

Three states (Iowa, Nevada and department of revenue, with the right Las, Las, and Las were regulated, suc- Maryland denied right of access by to levy against all highway depart-

#### **Highway** construction

Kansas expedited defense-road

January 1, 1942 • ENGINEERING NEWS-RECORD

projects by repealing a law requiring property owners. Maine assumed the trailers used in transporting logs.

New York voters in November apclared to be an emergency project and providing camping grounds. proval of the governor, without competitive bidding.

#### Streets and secondary roads

Alabama provided for allocating gasoline tax revenues to municipalities for road and street purposes. Arkansas added 184 miles to its state percent of the cost of maintenance of road mileage by a provision that the streets connecting main highways. highway department may take over or construct highways extending into or through cities of 2,500 or more population that are not now included in formity in limitations of size and roads that are not a part of the state length between first and last axles of a system. Connecticut increased its an- vehicle or combination. California town roads by an additional \$1,000,- 18,000 lb., but reduced the wheel

20 days between submission and ap- cost of certain activities for which the Texas limits vehicles having low-presproval of bids for highway work. towns have had to contribute (main- sure tires to 18,000-lb. axle load, Maryland will vote in November, tenance of main and third-class roads, 9,000-lb. wheel load, and 650 lb. per 1942, on a constitutional amendment snow removal and cutting of bushes); inch width of tire; for high-pressure permitting the State Roads Commis- an increase of <sup>1</sup>/<sub>2</sub>c. per gallon in the tires, the figures are 16,000 lb., 8,000 sion to enter upon private property gasoline tax is to cover this additional lb. and 600 lb., respectively. Tennesand proceed with road construction, expense. Maryland revised the law for see raised the gross weight from 24,in case of emergency, before institut- distributing funds to the counties to 000 to 30,000 lb., subject to 16,000-lb. ing condemnation proceedings. provide for a cheaper type of secon- axle load limit. dary roads that would not have to proved a constitutional amendment meet the state's standards and would to use \$60,000,000 of the state's not become part of the state highway

its highway commission to pay 50

#### Load restrictions

Progress was made toward uniments, while Kansas created a state department of civil service. Minnesota the state system. California amended weight of vehicles. California, Ingranted civil-service status to county its highway laws to permit the use of diana, Maine, Texas and Washington highway engineers. North Dakota state highway funds for work on mili- adopted a formula already in use by recognized the disadvantages of its tary roads, whether or not they are other state laws in which the gross low salary scale for engineers, and a part of the state system. This is con-weight of vehicle W equals C (L+ authorized an increase not to exceed ditioned on the United States agreeing 40); C is a coefficient, usually 700, to reimburse the state for work on but 800 in California, while L is the 20 percent. nual appropriation of \$3,000,000 for raised the axle limit from 17,000 to City Services on Map When taxpayers of Sacramento; 000, which is to be allocated on the limit from 10,000 to 9,500 lb., to inbasis of mileage of unimproved roads. sure a better weight distribution. In- Calif., want to find an obscure city Indiana added \$3,000,000 to its diana makes the axle load limit 18,- office or wish to locate an unfamiliar allotment of highway funds to cities 000 lb.; wheel load 9,000 lb., and not park they can refer to a special map and counties. Iowa authorized its to exceed 800 lb. per inch of tire prepared for them by the city manhighway commission to enter into width between the flanges. North Da- ager. The map, pocket-size, shows agreements with municipal authorities kota has a new truck licensing law the location of all buildings and propas to the construction and cost distri- with limits of 40,000 lb. gross weight, erties through which municipal servbution of extensions of primary roads 550 lb. per inch of tire width, and ices are provided, according to the through municipalities. Kansas has 18,000-lb. axle load. Ohio also adopts International City Managers' Assoauthorized cities to improve federal the 18,000-lb. axle limit. Oregon ciation. A municipal calendar and or state connecting links, and to pay adopted the above formula, with a co- other pertinent facts about the city for the right-of-way by levy upon the efficient of 650 if wheelbase does not and its government are printed on the entire city instead of upon adjoining exceed 18 ft. and 750 for trucks with folder.

ENGINEERING NEWS-RECORD • January 1, 1942

#### Administration

Indiana created a commission of grade-crossing funds for road-build- system. Minnesota counties will re- 20 members (including the members ing purposes, half for construction of ceive about \$1,200,000 from an addi- of the state highway commission) to highways and half for parkways and tional 1c. gasoline tax, in place of formulate a complete highway buildmain park roads. New Mexico author- revenue lost by repeal of a former ings and maintenance program, with ized the issue of \$4,000,000 in high- property levy. North Carolina in- means of financing it; a report is to way debentures for construction and creased its annual appropriation for be made by November, 1942. Utah improvement of state roads, including city streets from \$500,000 to \$1,000,- created a department of engineering the incidental engineering costs. 000. Utah added 255 miles of country to take over the duties of the State North Carolina provided that road roads to its state system and required Road Commission, state building contracts within a radius of 30 miles the State Road Commission to assist commission and state engineer, exfrom any national defense project de- in building roads into scenic centers cept that this engineer continues independent in the administration of water may be let by negotiation after three Vermont provided greater state as- rights. The new department will be days' advertisement and with the ap- sistance to towns. West Virginia has under the supervision of an engineercontinued until July, 1943, its 1c. per ing commission, which will be assisted gallon gasoline tax for surfacing sec- by an advisory council; the director ondary roads, and has authorized of highways must be a "practical highmunicipalities to improve streets way engineer." Washington has prothrough agreements with federal gov- vided for an advisory highway comernment agencies. Wyoming permits mission, but it is stated that this makes no change in the duties of the director of highways.

> Alabama created a merit system covering the various state departments, and Indiana created a similar system for engineers in all state depart-

## Builders Outline St. Georges Bridge Plan in Trade Paper

Unusual engineering features of the new St. Georges Bridge over the Chesapeake and Delaware Canal are explained in two articles in a tecent issue of the Engineering News-Record.

"How St. Georges Tied Arch was Erected" was written by William H. Bruce, resident engineer, and Charles U. Kring, assistant resident engineer, both of Parsons, Klapp, Brinckerhoff, and Douglas, New York, consulting engineers with the Army engineers on the bridge.

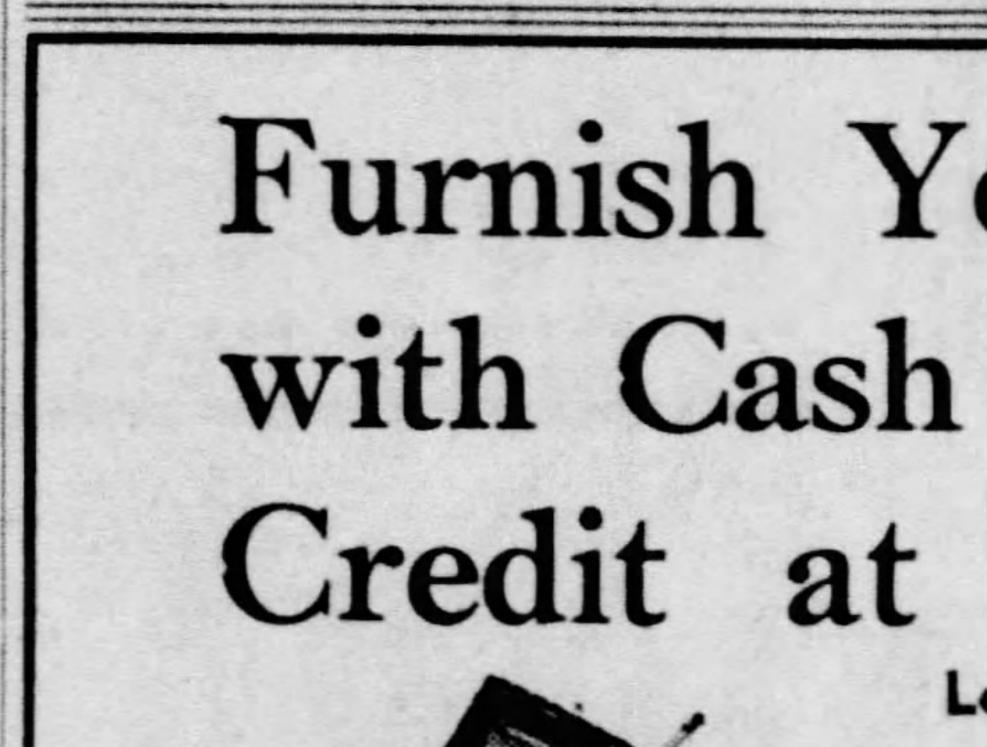
#### Engineer Is Author

Maurice N. Quade, structural engineer of Parsons, Klapp, Brinckerhoff and Douglas, wrote an article, "Tie Dominates Rib in St. Georges Arch."

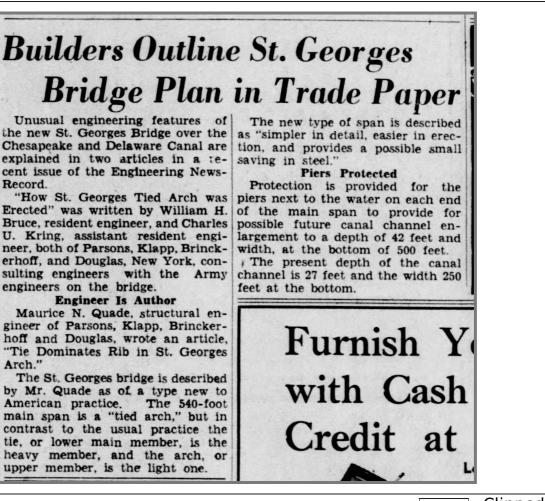
The St. Georges bridge is described by Mr. Quade as of a type new to American practice. The 540-foot main span is a "tied arch," but in contrast to the usual practice the tie, or lower main member, is the heavy member, and the arch, or upper member, is the light one.

The new type of span is described as "simpler in detail, easier in erection, and provides a possible small saving in steel."

Protection is provided for the piers next to the water on each end of the main span to provide for possible future canal channel enlargement to a depth of 42 feet and width, at the bottom of 500 feet. If The present depth of the canal channel is 27 feet and the width 250 feet at the bottom.



**Piers** Protected





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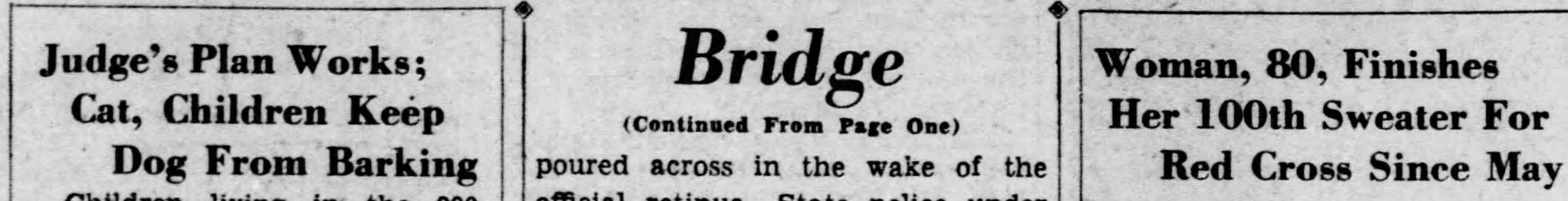
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## They Officiated at Formal Opening of New Span



Officiating at the formal bridge opening were, left to right: Governor Bacon, W. W. Mack, chief engineer of the highway department; Colonel Vaughan, and Mr. duPont.



Children living in the 900 block Elm Street and the Chesapeake Bay Retriever of Frank Boruta, who lives in the same block, have finally become friends.

Boruta was before Judge Henry R. Isaacs in Municipal Court on Jan. 17, faced with a charge of harboring a barking dog. His neighbors had preferred the charge.

Judge Isaacs suggested Boruta get a cat companion for the dog. Last Saturday Boruta reported to the court that the cat brought some improvement, but not enough to suit his neighbors. The court then continued the case until today with the suggestion that the dog and children get on friendly terms.

Today Boruta reported to the court that the dog and children had finally reached friendly terms and the case was dismissed.

#### **Brady-Reichners** Team Wins Acme Bridge Party

First honors in the duplicate bridge games sponsored by the women's committee at the Acme Bridge and Tennis Club yesterday went to Mrs. Frank J. Brady and Mrs. Lew Reichers with a score of 49 points. Mrs. Lillian Lynch and Mrs. Francis J. Durant were second with 46 points. Mrs. William Grady and Mrs. Joseph Nesbitt were third, 451/2 points. Others taking part in the games were Mrs. Norman Frick and Mrs. James McCrone, Mrs. Horace Ridings and Miss Theresa Smith, Mrs. Wil- engineers will be in charge of meeting of this committee Thurslard Ryan and Mrs. Gerald Doherty, maintaining it. Today's ceremonies day night. Mrs. Catherine Mulderick and Mrs. merely dedicated it to public use. Thomas Pritchard, and Mrs. Warren The new bridge completely over- Wyanoke Craft of Kennett Square, E. Young and Miss Frances Grady. shadows the town of St. Georges. Pa., Monday night when Mrs. Ada Strobridge were hostesses. Mrs. Frank town limits. The site of the old maid, and 40 members of Wynema Hickman and Mrs. Francis J. Du- bridge is 50 yards to the east of rant have been named hostesses for the new one-mile span. next week's games. Frank J. Grady won the honors in monies were entirely in the hands night by the propagation committhe mid-week session of duplicate at of state officials, the army and the the club on Thursday night with a contractors who worked on the score of 35 points. Mr. James May- structure. hart and Mrs. James McCrone were second with 30½ points and Mr. Some 35,000 yards of concrete Frank J. Brady and Mr. John F. and 7,500 tons of steel went into Taylor were third with 30 points. Others taking part were Mr. and Mrs. James Moore, Mrs. Francis J. Durant and Mrs. Lew Reichers, and Mr. and Mrs. Stuart Strobridge.

official retinue. State police under Lieut. Roger P. Elderkin had a momentary traffic problem on their hands directing the civilian motorists around the official party.

"It is a magnificent bridge and the Army engineers have done an excellent job, one that Delaware is well proud of," said Governor Bacon.

"Delaware appreciates the difficulties that had to be overcome in completing this structure," he added.

The bridge, major link in Delaware's highway system, replaces a lift span that was destroyed when a ship struck it Jan. 10, 1939.

Well-Timed Ceremonies The ceremonies were well timed, Slates Visits with the official party leaving the Slates Hotel DuPont at 10:30. In addition Governor Bacon, Colonel to Vaughan and Mr. duPont, this group included Col. Charles H. Gant, the Governor's aide; J. Francis Blaine, Elmer C. Taylor and G. Lester Daniels, Levy Court Commissioners; Samuel J. Reeves of Bryn Mawr, Pa., president of the Phoenix Bridge Company, which constructed the span; Harry B. Eaton, a member of the New Castle County airport ocmmission.

Barriers were still across the northern approach as the cars bearing the official party drove around a long line of traffic waiting to cross. Governor Bacon and his attendants posed for photographers at the base of the approach before the motorcade started to cross.

Soldiers On Guard

Mrs. Elizabeth P. L. Bell, 80 years old, established a record for Kent County Red Cross volunteer workers this week, when she completed knitting her one hundredth sweater since last May, all of which she donated to European War sufferers.

Mrs. Bell, who is a native of England, is one of the most active women in Kent County in her activities for European war relief nothwithstanding her advanced age.

# Staff of Lodge

Sachem, Aides, Will Attend Five Council Conclaves; Schedule Is Announced

Visits will be made to Red Men's councils by Great Sachem Russell C. Mayo and his staff, accompanied by Deputy Great Pocahontas Lillian Messick of Newark and a member of Mineola Council No. 17, on Feb. 11 to Mineola Council, Feb. 16 to Leola Council No. 14 at Union, and to Yonah Council No. 15, Bear, Feb. 19.

Great Sachem Mayo and his board of great chiefs will visit Wawaset Tribe No. 9 on Thursday in Red Men's Hall, 517 Shipley

#### **New Air Regulations To Take Effect Feb. 15**

All airports which fail to furnish a 24-hour armed guard will be closed by new Civil Aeronautics Authority regulations announced yesterady.

The new regulations, which govern both flight and basing of aircraft, provides that airplanes not under guard must be immobilized by removal of the engine, a wing, or both.

Soldiers with bayonetted rifles Street. patrolled the main span and two top of the bridge were grim reminders of war.

The bridge is "army property" Colonel Vaughn explained and the

The town itself arranged no for-Mr. Joseph Foreacre and Mrs. mality for today, and the cere-

#### Largest 'Tied Arch'

1.22

the making of this bridge, probably the largest "tied arch" crossing in the world. And a year and one half into the construction, started in July of 1940 with finishing touches being applied this week.

Parsons, Klapp, Brinckerhoff and Douglas, New York engineers who designed the span, said that the "tied arch" bridge is constructed in Europe but not generally in this country.

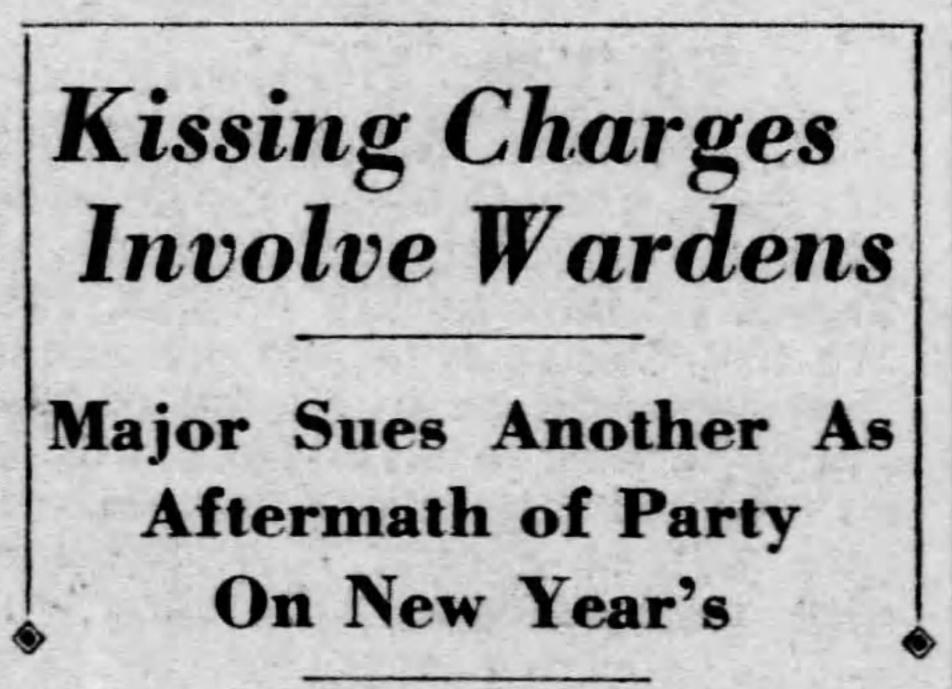
The builders were the Penker Construction Company, which constructed the foundations; Phoenix Bridge Company, steel work; George and Lynch, approach roads, and Corbetta Construction Company, decks.

The actual span over the canal is 540 feet in length and the navigation clearance is 135 feet. The opening of the span recalled the grimmer side of the story that had its starting Jan. 10, 1939. On that day the S. S. Waukeegan ran wild with a broken steering gear and rammed into the northern pler of the old lift bridge, carrying two men to death in the twisted wreckage.

The annual donation day for the sandbagged shelters perched at the Red Men's Fraternal Home at Newark will be held tonight at 8 o'clock under the auspices of the propagation committee. Plans for the annual banquet will be made at a

New officers were installed in the Mrs. Lillian Lynch and Mrs. Stuart with approach entrances at the M. Cooper, deputy state chief dairy Craft visited the group.

> Mrs. Gertrude Fleming is general chairman of a card party to be sponsored in Red Men's Hall Friday tee. Mingua Tribe will hold a similar party in the hall Feb. 9, with Arthur Purse as chairman.



NEW YORK, Jan. 31 (AP).-The question of who-if anyone-was kissed by whom at a New Year's Eve party of air raid wardens in the Bronx reached State Supreme Court today with the filing of a \$150,000 damage suit against a fellow warden by Maj. Frank Knowles, who charged false arrest. imprisonment, and defamation of character. The defendant was Ernest R. Rathkopf, who had said Major Knowles' conduct at the party was unbecoming a civilian desense officer. He declared the major went on a kissing spree. Knowles announced that he would resign his post today although he was cleared by three other zone wardens after a "court martial" proceeding in a Bronx police station on Rathkopf's charges. Rathkopf said he brought the charges after Knowles, his superior, told him his services were no longer wanted.

Until now, the C. A. A. has defined "immobilization" as removal of an essential part such as a wheel, aileron, or propellor.

In addition to obtaining permission from the Interceptor Command for all cross-country flights, pilots will be required to "sign out" with officers at the home airport, and telephone ahead to their destinations, telling when they expect to arrive. At the conclusion of a cross country trip, the pilot will be required to telephone or telegraph to his home port, stating time of arrival at his destination.

#### **300 Attend Supper**

an oyster supper of the Democratic a detour. Property assessments League of Delaware in its quarters dwindled. at 610 French Street last night. U. A year or so ago St. Georges S. Rep. Philip A. Traynor was discarded its charter; local govamong the guests.

Tighten Curb it Buying Habit

2. Fears in government quarters that Congress will fail to exact all of the \$9,000,000,000 in additional taxes asked by President Roosevelt thus leaving "excessive" purchasing power in the hands of the public.

#### Purchases Reduced

3. Curtailment of the sales of new automobiles, the largest single installment buying item, which has left an overly-large amount of available credit in the hands of the pub-

#### Town Disrupted

With the old lift bridge gone. St. Georges was a town divided. Community communications were disrupted; an automobile trip from one end of the town to the other, once a matter of a few hundred More than 300 persons attended yards, became a 10 mile trek over

ernment was no longer needed.

Delaware's highway system was completely awry with the bridge out. Getting to down state sections meant a trip over detours. State police had an extra job on their hands policing these by-passes, and accidents took a decided jump.

#### Three Years, Three Weeks

Three years and three weeks, to the very day, Delaware wrestled with these complications. The consternation was the whole state's but St. Georges was hit in particular. Even with the completion of the span the canal town fares poorly. The high type structure completely overshoots the town and St. Georges' main street is now a dead end stretch.



## Town Set for Ba States to Save

HUDSON, Wis., July 31 (AP) .-Hudson-population 2,987-seethed with indignation today and girded for battle against the states of Minnesota and Wisconsin, highway officials and congressmen from both states, trucking companies and bus lines to save its municipally-owned toll bridge.

The fight started a year ago when the two states moved for a new free bridge at or near Hudson. It reached a climax Wednesday in a Washington hearing on a bill authorizing the states to build the bridge.

#### **Bottleneck Charged**

At the hearing before a Senate commerce subcommittee. Hudson



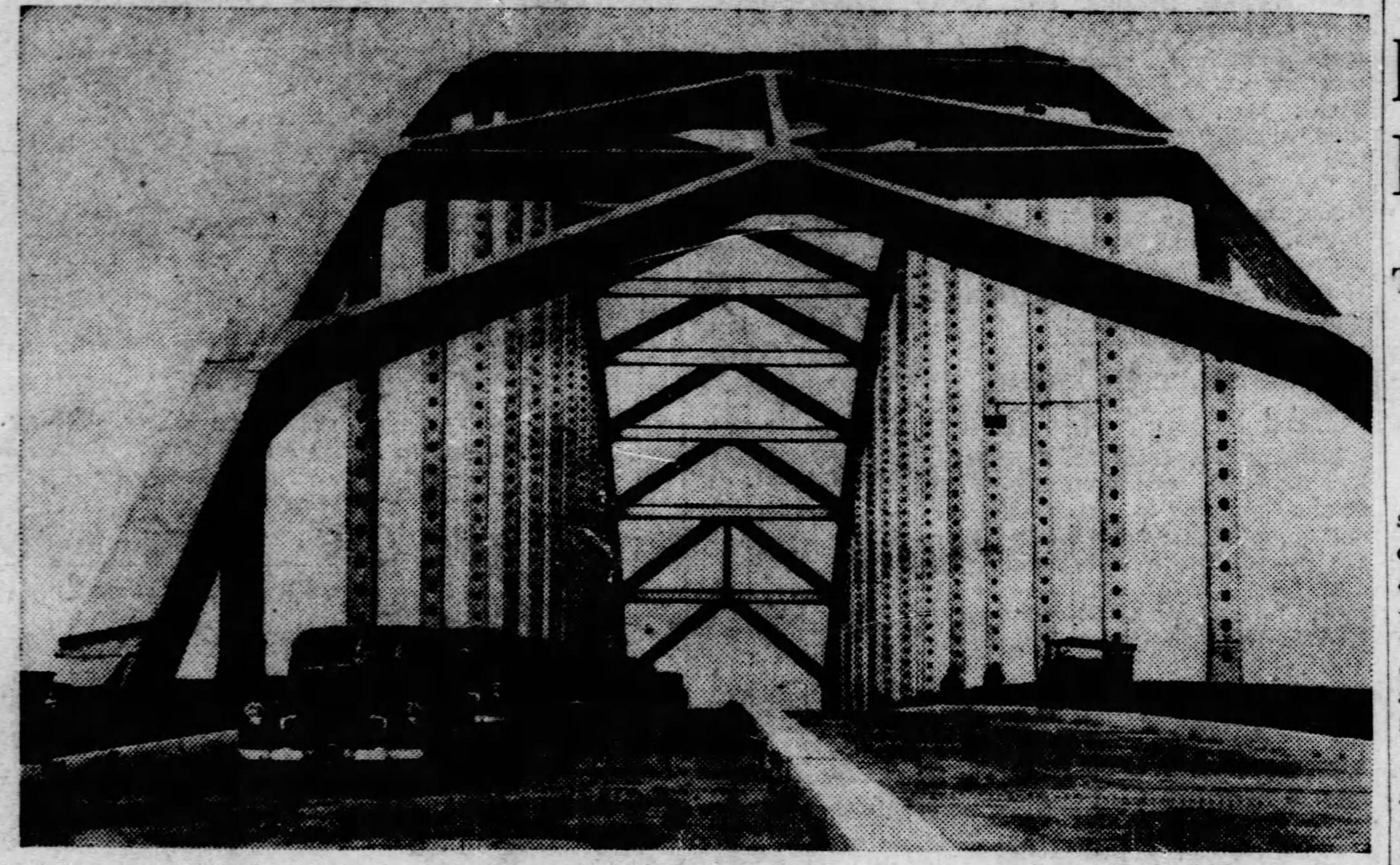


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DelawarePublicArchives Thu, Mar 16, 2017

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## Governor in First Car to Cross St. Georges Bridge



By Staff Photographer.

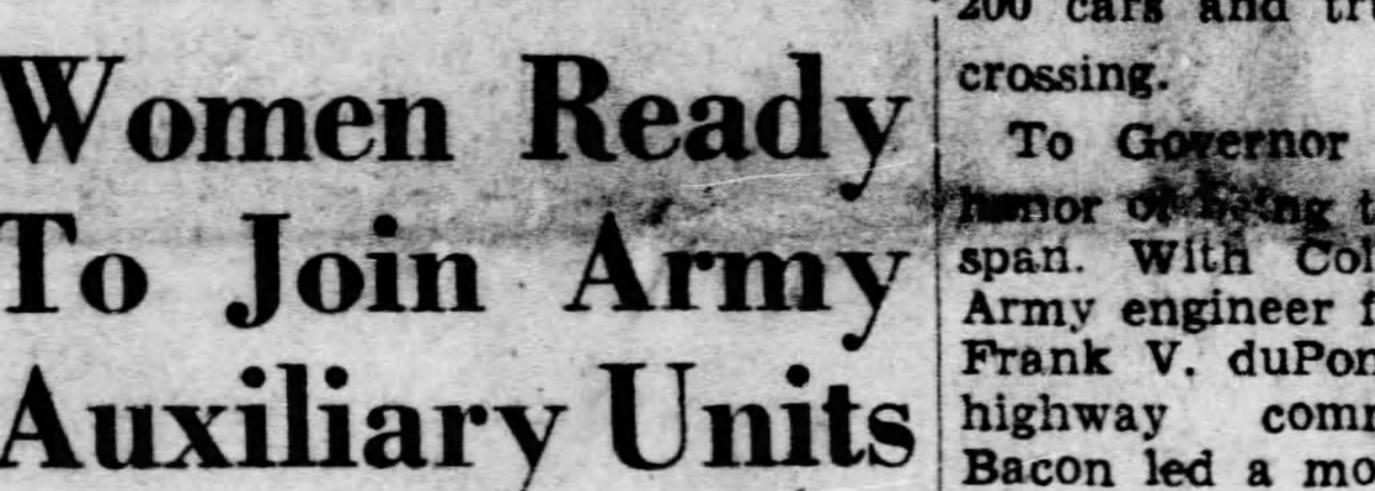
First to cross the new St. Georges bridge were Governor Bacon, Col. H. B. Vaughan, and Frank duPont, whose automobile is seen as it reached the crest.

## St. Georges Bridge Opened; War's Impact Bacon First to Cross Span Is Discussed

## Unpretentious Ceremonies, 'Because of the Gravity Of By Publishers These Times,' Mark Dedication; 200 Cars Use Structure During First Ten Minutes

St. Georges Bridge, a \$2,500,000 structure that spans the Chesapeake and Delaware Canal at the town of St. Georges, was opened to the public this morning with Gov. Walter W. Bacon officiating.

The ceremonies, lacking pretentions "because of the gravity of these times" took five or six minutes and at once the public was making good use of the new structure. Within 10 minutes after the formal opening at least



200 cars and trucks had made the

Bacon went the monor of the first to cross the span. With Col. H. B. Vaughan, Army engineer for this district, and Frank V. duPont, chairman of the highway commission, Governor Bacon led a motorcade on a round trip over the bridge. Less than 100 of the town residents turned out for the short ceremonies which had its start at the northern approach at the town's limit. Whistles on two tugboats, an army engineer tender and a cargo ship plying the canal, sounded a salute as the official cars reached the main span of the bridge. Down to the southern approach end went the corps of cars and then back across to the starting point. It was just 11 o'clock-official opening time-when the Governor's car first reached the peak of the bridge. Motorists, some of whom had been waiting for more than an hour.

**Loss of Advertising Profits** Strikes Press; Weeklies **Consider Rise** in Price

. The effect of the second World War, particularly upon weekly newspapers, was thoroughly discussed today by publishers of the Delmarva Peninsula at the joint annual meetings of the Del-Mar-Va and Maryland Press Associations in the Hotel DuPont.

Loss of national and local advertising as a result of war-time conditions was particularly in the focus of the discussions.

Emphasis of the entire meeting

They Would Be Paid \$21 Monthly, Bill to Create Corps Will Pass House

WASHINGTON, Jan. 31 (INS).-Thousands of American women are ready to enlist in the Women's Auxiliary Army Corps at \$21 a month, Rep. Edith Nourse Rogers (R-Mass) reported today revealing that she is being deluged by mail from the ready volunteers.

Sponsored by Mrs. Rogers, and unanimously approved by the military affairs committee, the bill creifammad maman'

(See BRIDGE-Page 10)

was also placed on problems of circulation, advertising, and the handling of public information released through federal, state, and municipal authorities.

#### Price Rise Studied

Proposals to increase the annual subscription rate of weekly newspapers were studied as a means of supplying revenue which has been lost since large manufacturing companies, formerly national advertisers in weekly papers, have in many cases discontinued weekly newspaper advertising altogether. It was suggested that an average of 50 cents a year be the increase in

(See PUBLISHERS-Page 4)

**Red Cross Working** With A. E. F. in Ireland BELFAST. Ireland. Northern





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By 2,840 for State Senate

Republicans today held veto control over both houses of the Legislature with the victory of Edward Abrahams, Jr., over Sigmund Schorr, Democrat, in a special election in the Second Senatorial District of Wilmington yesterday.

They already held that power in the House by a three-fifths majority and Abrahams' victory gives them the same three-fifths margin in the state Senate, enough to override any veto by the Governor.

Abrahams, polling 9.311 votes Schorr's 6.471 for a majority of 2,840, will take the Senate seat held by the late Daniel E. Kelly, Demo-

The Republicans will control the Senate 11 to 6, and the House 21 to representing the National Crusaders 13 with a vacancy yet to be filled of America, was the first witness to due to the recent death of Dr. Jo- testify in opposition to Frankfurseph Roop Smith, Democrat, of ter's confirmation by the Senate. He Marydel.

position to carry out its entire leg- question." islative program.

Party leaders have indicated there will be no "ripper" legislation, al-

(Please Turn to Page 4, Column 3).

Mystery Noise Upsets Brooklyn Blame Rumblings On Everything - Even The Dodgers -----NEW YORK, Jan. 10 (P).-The

"case of the big noise"—a recurren rumbling from the skies that startled hundreds of Fatbush residents-left Brooklyn police with an unsolved mystery today.

Still remembering the Orson Martian Welles broadcast. Brooklynites crowded into street and listened to the strange roaring emerging from a cloudy sky at irregular intervals from 10:45 to 11:30 o'clock last night.

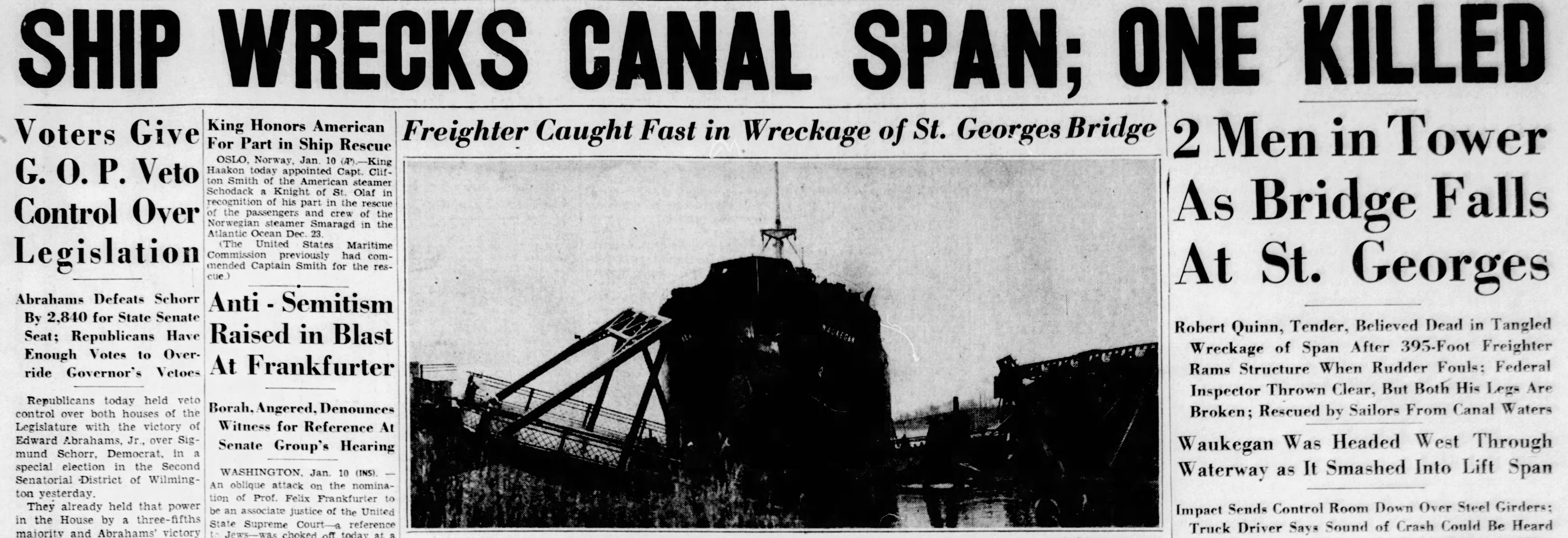
Scores phoned police. and squad cars cruised about checking reports of explosions and earthquakes.

Mystified, the police investigated and discarded rumors that it had been caused by:

(a) A roller skating rink; (b) An earthquake; (c) A blast: airplane; (e) A windstorm; (f) amateur radio operator: (g) A lion in the Brooklyn Prospect Park zoo; (h) A tidal wave; (i) The Brooklyn Dodgers.

Scientists at Fordham University said no earth shock or explosion had registered on the seismograph there. The weather bureau reported no thunder.

A young girl said the noise sounded like "surf dashing against a rockbound shore." That only obscured the issue further.



Atlantic Ocean Dec. 23.

State Supreme Court-a referent Jews-was choked off today at a Senate hearing with a wrathful denunciation by a leading Republican -Senator William E. Borah of Idaho.

Before a crowded chamber in which a Senate judiciary subcommittee started hearings on President Roosevelt's appointment of the famous Harvard liberal, the Idaho senator demanded that all allusions to semitism be omitted.

Collis O. Redd of Washington, started to quote from newspaper The victory places the party in a articles on the so-called "Jewish

Senator Borah interrupted him immediately.

"Are you here opposing Frankfurter on the ground that he is a Jew?" the senator demanded "No," Redd replied in an abashed

"Then," Borah snapped, "why

(Please Turn to Page 4, Column 2).

#### **Steele Plans Revision Of Senate Committees**

President Pro Tem David Wilmer Steele of Ocean View announced today he intends to revise the list of State Senate standing commitas a result of the election Edward Abrahams, Jr., in the Second Senatorial District yesterday.

He pointed out that if the committees were not revised, the representative of the Second Senatorial District of this city would not be on

## Grand Jury Acts In Probe of 14 **Political Leaders**

HARRISBUR , Pa., Jan. 10 (AP). -A Dauphin County grand jury, resuming its investigation of campaign charges against 14 state Democratic leaders, took up today the problem of what portion of the charges it would consider next.

gravel was used on highway projects tration. in Erie and Warren Counties

The gravel case brought from the Jury Saturday its first indictment. The true bill accused . David monwealth and state Democratic House and Senate.

spector, was thrown clear of the twisted girders, but both his legs were broken.



Engineers say it will take two or more weeks to remove the twisted structure of the bridge from the canal. The picture at the right shows the center span in the water. At the left is the Waukegan aground behind the bridge pilings.



House Committee Due To Slash Appropriation, But Hopkins' Victory Is Seen

WASHINGTON, Jan. 10 (AP) .-- Congressional economy advocates and critics of "politics in relief" moved Bonnet. The jurors already have indicated rapidly today toward an initial Following a cabinet meeting at D. into a report that sub-standard over the Works Progress Adminis-

> heightened on Capitol Hill by the Bonnet told his colleagues he had L. fact that it will afford the first test received assurance of "the complete

chairman, of blackmail, conspiracy On the House side, the issue was

# By Italy for Share of Empire

PARIS, Jan. 10 (AP).-An agreement between France and Britain to reject any Italian territorial demands for a share in the French empire was announced to the French cabinet today by Foreign Minister Georges

they would like to continue looking showdown with Roosevelt forces the Elysee Palace, shortly before Roosevelt Plans British Prime Minister Chamberlain and Foreign Secretary Viscount Halifax were to reach Paris on their Interest in the outcome was way to Rome, it was disclosed that

Lawrence, secretary of the com- of the President's strength in both accord of the London cabinet with the French point of view." This, Bonnet said, had been conauthoritatively today, has decided age limit for veterans. to "cheat and defraud" the state, drawn by Mr. Roosevelt's request for veyed to him last night by Sir Eric tentatively on a Caribbean cruise (The Delaware Department of

Her prow aground, the government-owned freighter Waukegan is caught fast in the wrecked superstructure of St. Georges Bridge, with one man missing and another seriously hurt after the ship crashed into the bridge this morning. Highway and canal traffic was disrupted. Robert Quinn bridge tender, was believed drowned in the control house (to the right of the boat) which fell into the water. William Oakes, government in-



French Cabinet Told London in Agreement With Daladier Government to Reject Any Demand



## Vets Will Press For Enactment Of Civil Service

-By Staff Photographer.

Legion and V. F. W. Seek Preference for Former Soldiers in State Jobs

The Delaware American Legion will sponsor a bill in the legislature, setting up a civil service or merit system for state employes.

The legion's executive committee Ruler With Bomb Is at a meeting in Selbyville last night **Related** in London approved a report of the legislative committee, recommending the bill. The proposed law would provide LONDON, Jan. 10 (INS).-The dipfor a five-point preference for vetlomatic correspondent of the Lonerans who apply for state jobs, a WASHINGTON, Jan. 10 (AP).- 10 per cent preference for disabled don Daily Telegraph reported today President Roosevelt, it was learned veterans and the waiving of any (that serious attempt has been made (Please Turn to Page 10, Column 5). against the life of King Carol of

# As Bridge Falls At St. Georges

Robert Quinn, Tender, Believed Dead in Tangled Wreckage of Span After 395-Foot Freighter Rams Structure When Rudder Fouls: Federal Inspector Thrown Clear, But Both His Legs Are Broken; Rescued by Sailors From Canal Waters

Waukegan Was Headed West Through Waterway as It Smashed Into Lift Span

Impact Sends Control Room Down Over Steel Girders; Truck Driver Says Sound of Crash Could Be Heard Three-Quarters of Mile Away; Quinn Believed to Have Been Killed or Drowned; Traffic on Highway Tied Up

A bridge tender was reported killed and the government inspector injured this morning when the 395-foot freighter Waukegan crashed into the bridge at St. Georges, toppling the steel structure into the Chesapeake and Delaware Canal.

The ship, its rudder fouled, smashed into the north tower, umbling the tower and the center span into the water. The two men were in the control house on the center span.

Robert Quinn, 45-year-old bridge tender of St. Georges, was believed trapped in the tower. His body had not been reccat noon today.

#### Man Thrown Clear

William F. Oakes, 45, also of St. Georges, was thrown clear of the control house, and was rescued by a rowboat from the Waukegan after clinging to the partially submerged structure for 20 minutes.

Oakes, with both legs broken, was rushed to St. Francis Hospital in Wilmington for an emergency operation.

Norman Sparks of St. Georgese had gone off duty as bridge tender only a few minutes before the acci- Canal Bridge

\$300,000 Damage to Bridge estimated at \$300.000.

Clement Vaughn, a U. S. surveyor working nearby said the vessel apparently went out of control su

Both anchors full speed astern, but the freighter on, jamming into the north

**Boat Bound For Baltimore** crumpled, splash into the canal, 25 deep at that point. It pinned the up's prow. With it went the roadbed, lifted

(Please Turn to Page 10, Column 1

Attempt to Slav Carol Reported Iron Guard Plot to Kill

S300,000 Damage to Bridge Damage to the bridge was unoffi- Cost \$500.000; falling girder tore a hole in the bow Opened in 1926 of the freighter above the waterline.

> Federal Government Built Span to Replace Wooden Swing; Took 21 Months

The St. Georges bridge across the Chesapeake and Delaware Canal was constructed by the federal government at an approximate cost of \$500,000.

It was a vertical concrete and teel bridge, replacing an old wood swing span.

Construction was begun in July, 1924 and it was open to traffic March, 1926.

The bridge 450 feet long Was 205 feet long, was raised on cables from two 170-foot steel towers. The approaches were 120 feet long each. The lift span was raised by electric current with reserve gasoline apparatus in case of emergency. The mechanism was housed on the lift. The two steel 170-foot towers rested partly on the pier proper and

the canal end of the approaches. The bridge roadway was 24 feet





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(Continued.) like an elevator between the two

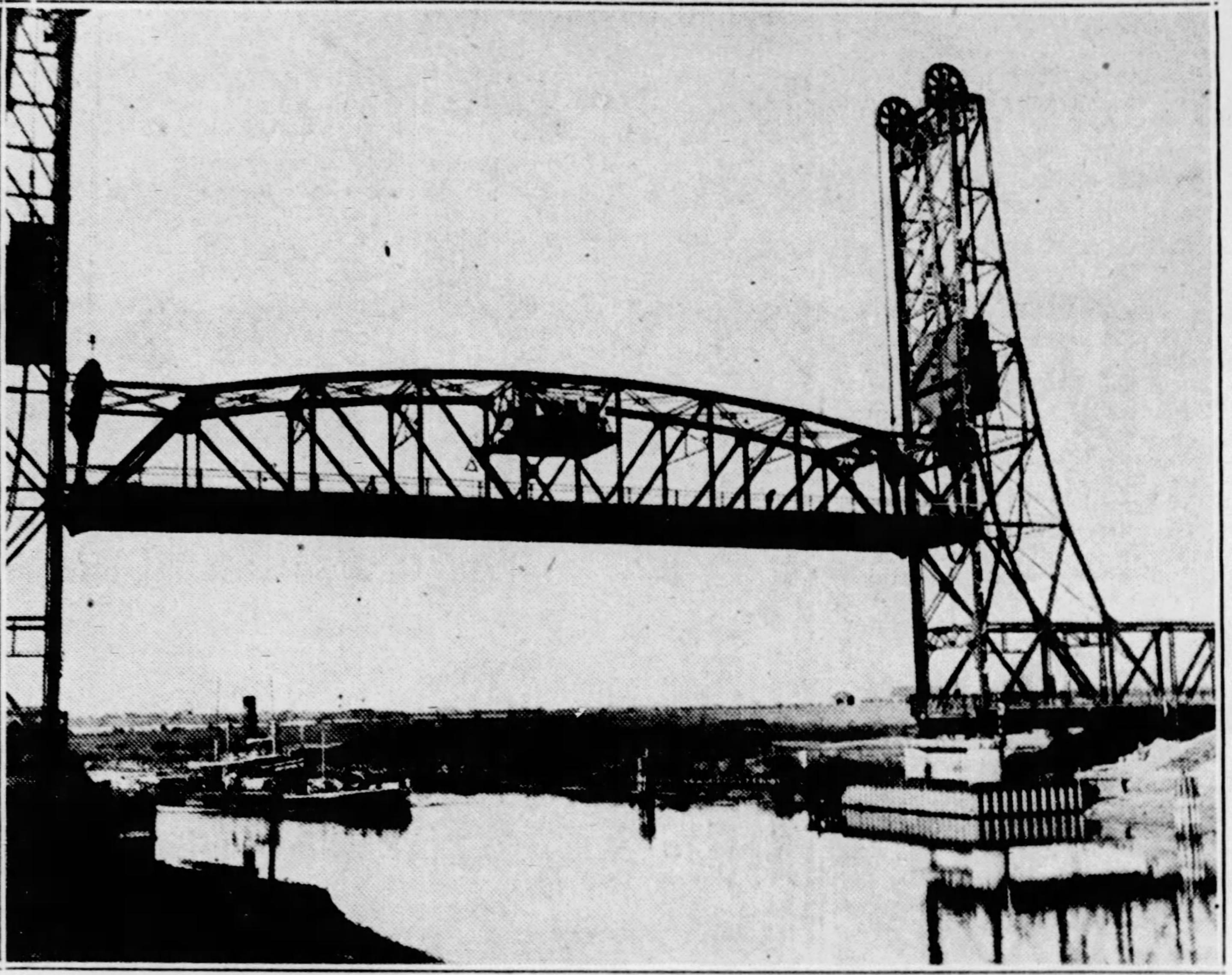
towers for ships to pass, and the top section with the control house. The Waukegan, commanded by Capt. John R. L. Reynolds and owned by the Maritime Commission, had entered the canal about dawn, and the accident occured at 8:30 o'clock. It was bound for London via Baltimor with a cargo of grain.

"there One evewitness declared was a terrible noise, and the whole seemed to collapse. The bridge structure fell on the forward part of the ship."

A truck driver said he heard the sound three quarters of a mile away. Quinn was believed either killed in the crash, or to have drowned as he struggled to free himself.

The two men in the control cabin plunged more than 150 feet down into the water. The bridge when lowered is 50 feet above the water level. Traffic on the DuPont Boulevard was forced to detour. Telephone communication was disrupted when a cable was apparently torn by the wreckage. **Three Investigations Launched** U. S. Anny Engineers declared that even "if we have luck it will be at least two weeks" before the waterway, one of the principal canals in the East, can be reopened for traffic. Four investigations loomed, one by army engineers, a second state police, the third by Coroner the Maritime Commission.

## St. Georges Bridge Before Crash



a block and a half from the bridge, tance between the steel towers was Fa said he was aroused by 'the terrific | 210 feet noise" when the accident happened. When construction was begun in He said he did not know what had 1924 cofferdams had to be constructhappened and rushed outside. Mr. ed to hold back high water during S Carrow is a juryman in the Court of the sinking of the piers. The pier -TGeneral Sessions. Frank W. Schroeder, clerk of the pleted by the Dravo Contracting coll House of Representatives, rushed to Company, and the steel bridge work

the bridge from his home in Dela- by the Phoenix Bridge Company. Lan ware City. He said that about 2,000 Harrington, Howard, and Ash of persons were there when he reached Kansas City, were the architects. pla the bridge. "It was a terrible In 1937-38 longer shore approaches ma sight," he said. "Both towers, the were built so that enlarging of the that lift and the superstructure were channel could be dong. steel."

#### Lester Eye-Witness

Claude N. Lester, president of the New Castle County Levy Court, who had just come out of Fred Sutton's store, about 275 yards from the bridge, said he heard the crash. "I looked in the direction of the bridge and saw streaks of fire, as the bridge went down," he said. 'When it struck, the water splashed more than 15 feet in the air. was a horriblie sight." Lester lives at St. Georges. Quinn is survived by his wife. Elizabeth, and two daughters. Odell a student at Ursuline Academy, and Mrs. Dorothy Lutz, who lives at his nome. Oakes is married, with eight children: William, a freshman at the University of Delaware; Francis student at H. Fletcher Brown Vocational High School; Alice. student at Ursuline Academy; Peggy. who attends Delaware City High School; Bobby, Dorothy, and Paul, pupils at Macdonough School, and Teresa, an invalid. His wife is Mrs. Teresa Oakes.

and other concrete work was com- sho rat

down. The abutment on the north The bridge was designed for the side was completely submerged, government by John Lyle Harring- said There was a mass of entangled ton of Kansas City, one of the cor outstanding engineers in the world. his Harrington arrived in Wilmington he last night on private business and registered at the Hotel DuPont.

#### **Delaware City Phone** Exchange Kept Busy

Officials of the Diamond State Telephone Company said two men were sent from the office here to Delaware City exchange today to assist operators there in clearing the traffic. The Delaware City exchange was flooded with calls after reports of the bridge's destruction. Company officials said they had no reports of their cables under the canal being damaged. The cables, it was reported, are buried in the dirt under the canal and were functioning normally.

Engineers said the bridge valued at about \$500,000.

Auto Traffic Detoured Automobile traffic will be forced to detour for months, it was said. The northern end of the span was a mass of twisted girders. The superstructure on the southern end was also twisted.

Replacement of the structure will in half. cost roughly \$300,000, said J. W.

St Georges bridge over the Chesapeake and Delaware Canal, as it appeared before crashed into this morning by the steamer Waukegan. This bridge was reopened to two way traffic Nov. 19, 1936, after having undergone extensive enlargement, in connection with the widening of the waterway.

way for about five miles north of

built under wartime construction by said rolling approaches-much as in treacherous and that it was flowing operated immediately when Oakes James F. Hearn and the fourth by the government in 1919. Its gross ferry slips-had been suggested for tonnage was 6,208. It had a draught all canal bridges, but the government did not see fit to construct of 17.5 feet.

them.

Georges.

**Town Divided** 

St. Georges found itself a town divided, with no passage across the river except by small boats.

An engineering employe said Henry C. Ray blocked off the highpontoon bridge might be thrown across the canal for foot and lighter traffic, to join the village now cut way of Summit Bridge and Middle-

town. Three blocks from the scene & Co. He said the superstructure in their home, while firemen made fast in the tangled wreckage, and struck the structure.

pened.

John A. Taylor, an eye-witness, declared he could feel the vibration of the collision where he stood The high towers of the bridge 300 feet away on Main S'reet in were visible for miles around St.

St. Georges. State troopers directed by Capt.

Folded Like Matchwork

"The tower folded up like match-I heard the whistle blow work. the crossing, diverting traffic by for the bridge to go up, and saw the boat approaching."

Taylor added he was paying lit-By early afternoon the prow of the tle attention until he heard

Doctors at St. Francis Hospital strongly when the accident hap- arrived there. His condition was "just fair."



#### (Continued.)

wide, with two sidewalks of four and one-half feet each. Steel plate treads were located on the roadway.

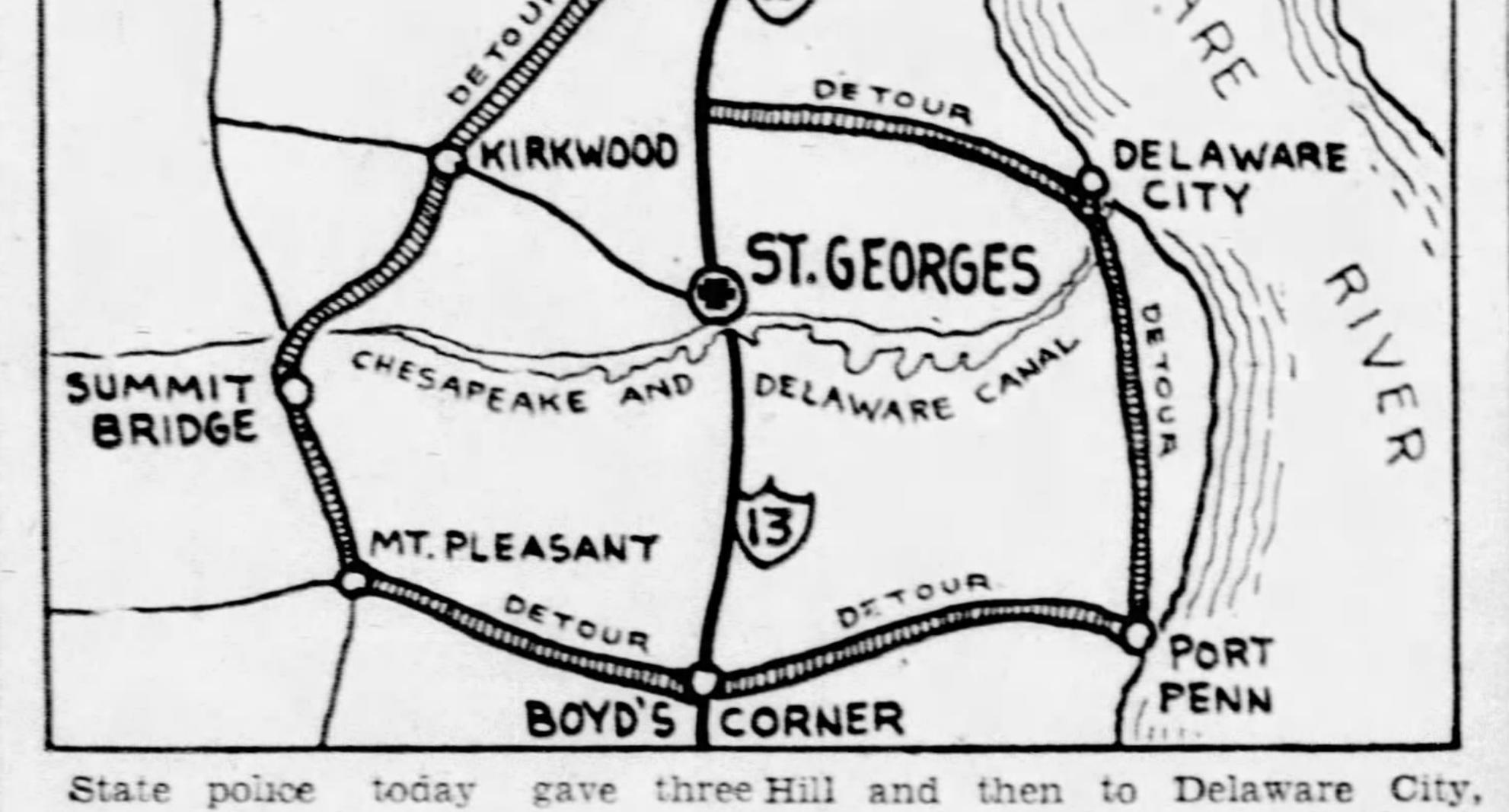
#### 138 Feet Clearance

The lift had a vertical clearance



Is your nose irritated-is it clogged with mucus-does your throat get choked with phlegm -are you losing your sense of taste, smell, due to a cold? Are your breathing passages corked up? Get a tube or jar of Mentholatum today. Wake up tomorrow morning with a clearer head and feeling like a new person. This soothing balm-when applied in the nostrils-vaporizes almost instantly. Its active comforting Fairbrother, engineer of J. A. Bader Quinn's wife and children waited & The lift had a vertical clearance vapors help break up the choking mucus, re-& Co. He said the superstructure in their home, while firemen made could not be salvaged but the abut- plans to drag the canal for his body.

ments and piers were still left. The company did major concrete work on abutments and piers in 1936. A counterbalance weight of 22 tons of steel and iron crashed down through the ironwork on the south- ern end of the bridge. The north- ern approach and center span were gone, with a noticeable sag in the southern approach. The Waukegan struck with such force that it moved the bridge 30	Captain of Boat Silent aptain Reynolds, leaving his ship at 12:30 o'clock, said he had no tement, but had "profound re- ts at the loss of life and injury the other man because of the ident." e conferred with Major C. W. lin, U. S. District Engineer, C. Meyers, resident engineer, and Y. Taggert, engineer of the sub- ce at Chesapeake City. ater he met with Lieut, Roger erkin of the state police and er policemen	Light rain and overcast skies gave a sombre note as crowds of townspeople flocked to both sides of the canal. Crowd of 7,000 Gathers Some 7,000 persons jammed the small town this afternoon. State police said that on the bridge at the time of the crash were Cap- tain Reynolds who lives in Balti- more; F. Leroy Taylor of the Dela- ware River Association of Pilots; Cadet William Ingram of Norfolk, Va., wheelman, and Horton H. Har-	a farmer, ran to the edge of the canal when the bridge toppled, and reached it just as Oakes came to the surface. Oakes kept calling for help until the rowboat reached him, he said, and then with Earl Schwartz, and Gilbert Carrow, Wilson assisted in taking him from the boat to a light delivery truck. State police escorted the truck in a dash to Wilmington. Judge Charles L. Terry, Jr., de-	I. ELME "A Superior F Coal, Koppers 29th & Boulevard	plers breath. Ask your druggist for Mentholate plers today. In jars or tubes. 304. Cepyright 1989,
canal bottom before water traffic can be resumed, engineers added. The steel-plated Waukegan was wor Detours for Motorists A	former employe of the Wiscon- Bridge Company, who had ked on construction of the span, round St. Georges	For bridge said it collapsed like a child's bridge set when stepped on. <b>Crumpled Like Tissuc Paper</b> "You'd think it was more like tissue paper than steel," one resi- dent told W. Lyle Mowlds of Dover, who was one of the first motorists stopped at the south side of the canal's bank after the accident. Veteran canal men said the tide in the vicinity of the bridge is	cident occurred, on his way to Wil- mington where he is sitting in the Court of General Sessions. John W. Carrow, who lives about "DADDY, COME LET THE WORMS WARM YOU!"	Pennsylvania I	SITURE STATES AND STATES STATE



alternate routes which may be used across the Reedy Point Bridge to to detour about the destroyed St. Port Penn, and turn right at Port Georges Bridge over the C. and D. Penn back to the DuPont Beulevard just south of St. Georges. Canal. Motorists from Wilmington can Motorists may also drive south to go south on the DuPont Boulevard Tybout's Corner, turn right to Kirkto State Road, bear right and go wood, right at Kirkwood, proceeding on route 40 to Glasgow, turn left at to Howell's School, where a left turn

Glasgow and go straight south to is made, then straight south to Sum-Summit Bridge, across the canal to mit Bridge, to Middletown, and Blackbird. Middletown and Blackbird.

Another route is from Wilmington Northbound traffic may use the down the Causeway to Wrangle same courses.

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