



Erection of two main spans by two derricks operating in opposite directions from mid-river pier.

Leonard Studio Photos

Spanning the Mississippi at Chester, Ill.

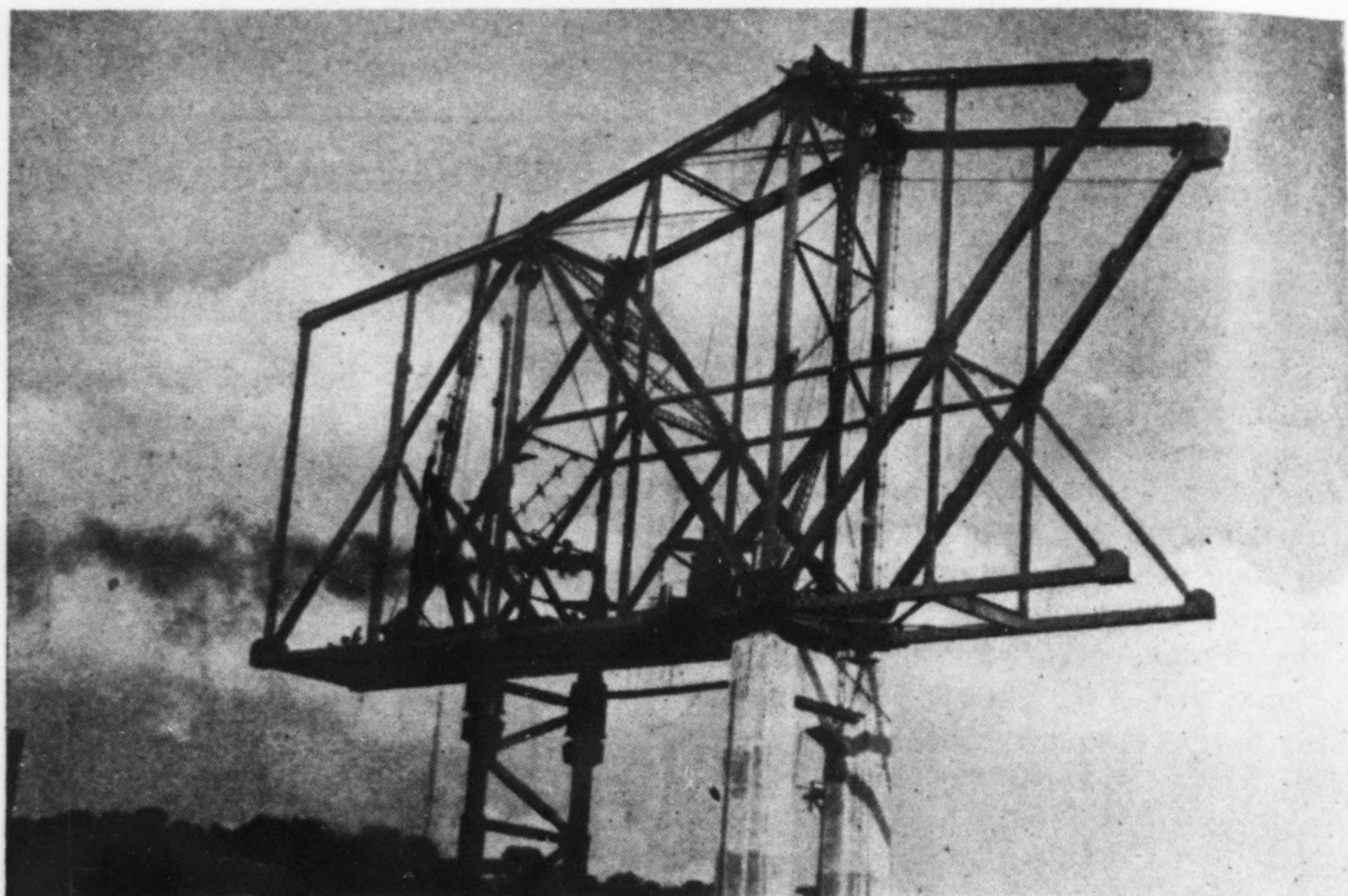
The highway bridge now under construction over the Mississippi River at Chester, Ill., to connect Illinois Route 3 and Missouri Highway 51, will be completed by spring. A 22-ft. wide roadway and two sidewalks will be provided.

Over the main channel a 1,340-ft. long continuous through truss unit of two equal spans is used. The trusses are 281½ ft. center to center and they are 100-ft. deep over the mid-river pier. This central unit is flanked at each end by a 500-ft. long continuous deck truss, while the Illinois approach is one 60-ft. I-beam span and the Missouri approach seven 60-ft. similar spans. Overall length is 2,826 ft. A clearance of 107 ft. is provided at extreme low water.

The mid-river pier and the Missouri pier supporting the main trusses were sunk with pneumatic caissons and built on rock 100 and 110 ft., respectively, below low water. The main pier on the Illinois side was constructed by means of an open cofferdam and rests on rock about 30 ft. below the surface. All piers for the Illinois approach are also carried to rock, but those for the Missouri approach rest on timber piling.

Superstructure erection

Main-span erection began at the mid-river pier and was carried out with a derrick barge and two derricks mounted on the bridge's floor system and proceeding in opposite directions. The first temporary falsework bent was located on the Illinois side at a panel length from the mid-river pier. The derrick barge then erected the two lower chords, the floor system and a portion of the trusses for the first Illinois panel. Next the two derricks, facing in opposite directions, were placed on top of the pier. After they had erected together the top chord sections over the central pier, erection was cantilevered in both directions with a balance maintained on the pier and the falsework bent. In all, three bents were required for the Illinois span, while one bent requiring a cantilever of 402 ft.—the longest on the job—was used for the Missouri side.



Later stage of work over central pier with erection balanced on pier and one falsework bent.

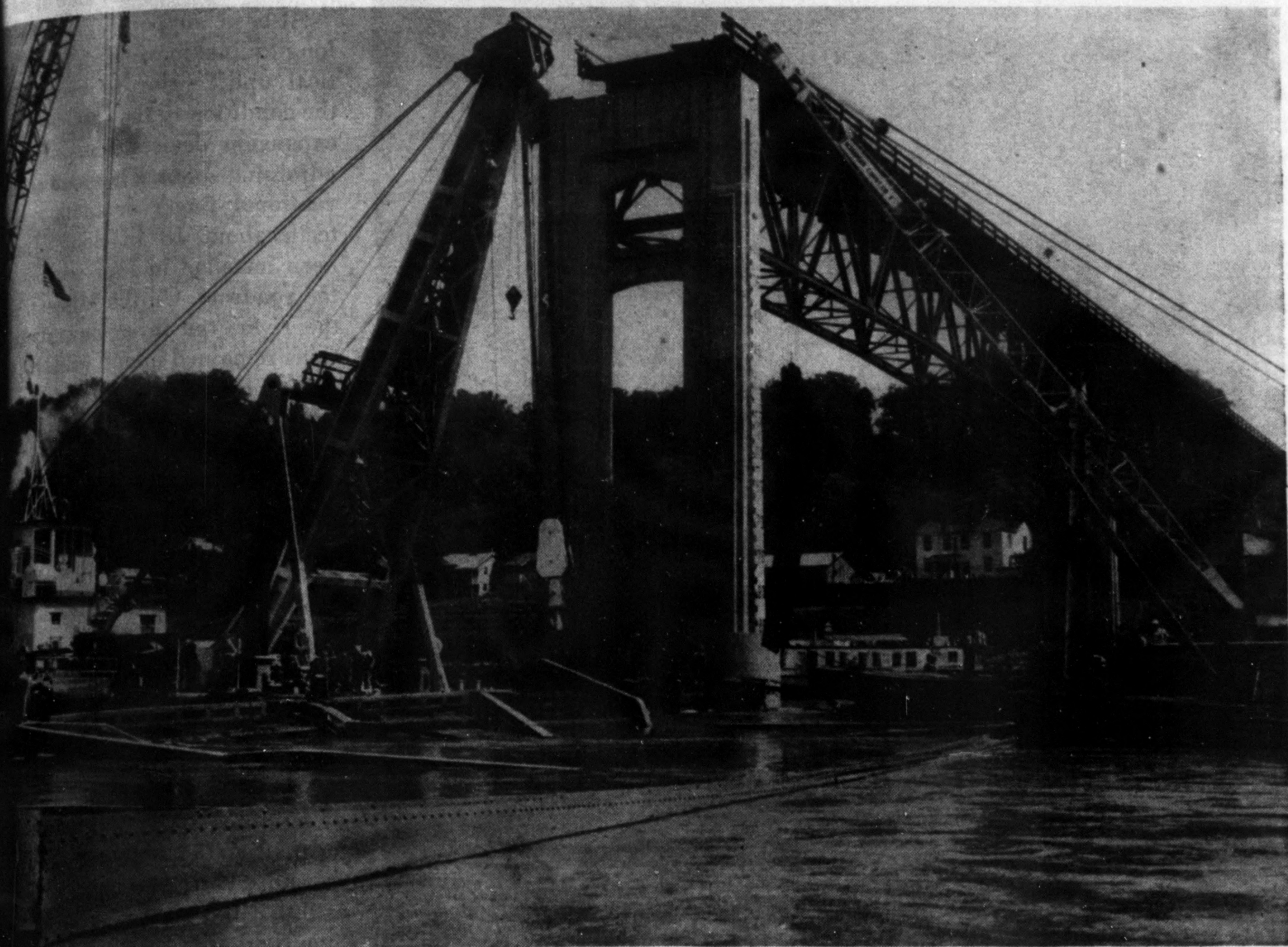


Two 670-ft. main spans fully erected. One falsework bent in each span is holding camber until riveting can be completed.

Each steel falsework bent rested on 40 timber piles 80-ft. long and of 40-ft. penetration. The piles were braced above water by bolted timbers and below with ¾-in. dia. steel cable. High water and swift currents occurred during the 3-month period while the bents were in place. A moderately severe 10-day flow of ice constituted a serious hazard to the

falsework during this phase of the construction work.

Construction, which started October, 1940, is financed by the city of Chester. Massman Construction Co., Kansas City, with a \$1,090,000 contract, is doing the foundation and superstructure work. Sverdrup & Parcel, St. Louis, are designing and consulting engineers.



Heavy hoisting equipment removes torn and twisted bridge trusses from the muddy Mississippi at Chester, Ill.

Removing a Bridge From the Mississippi

Contents in Brief—Heavy marine blasting removes 500 tons of the wreckage of the Mississippi River bridge at Chester, Ill., to provide a 300-ft. navigation channel. Salvage operations completed in 15 days and without mishap, despite unusual hazards. Reason for failure of bridge in wind still unexplained.

How does a bridge constructor remove a large river bridge, then turn around and remove part of it from the wreckage within a period of two years. This is what happened at Chester, Ill., when the same contractor who built a bridge over the Mississippi River there in 1942 recently removed 300 ft. of the two 670-ft. continuous spans which toppled into the river during a windstorm of tornadic intensity on July 29.

The owners of the bridge, the Chester Bridge Commission had the first responsibility for removing the collapsed truss spans. But as guardians of

continuous and uninterrupted flow of river traffic the Corps of Engineers, U. S. Army, had the task of expediting the removal of a sufficient length of bridge to restore full use of the navigation channel. Accordingly these two agencies met and agreed that the Army Engineers would remove 300 ft. of bridge on the Illinois side of the river. Despite the fact that the job promised to be dangerous, difficult and expensive (steel that cost $9\frac{1}{2}$ cents per lb. erected cost 5 cents per lb. to remove), speed was imperative.

Barge tows could use a channel 150 ft. wide and 9 ft. or more in depth

between the Illinois bank of the river and the pier which had supported the east end of the continuous trusses, but there was always the danger that it might be blocked. A salvaging contract, therefore, was negotiated, equipment was mobilized and removal operations of the part of the steel in the main channel were started Aug. 13. Work continued 12 hours a day (night operations being considered too hazardous), and fifteen days later the 300 ft. of bridge, containing some 500 tons of steel, was completely removed and piled on the Illinois shore a short distance below the site.

Manner of failure still obscure

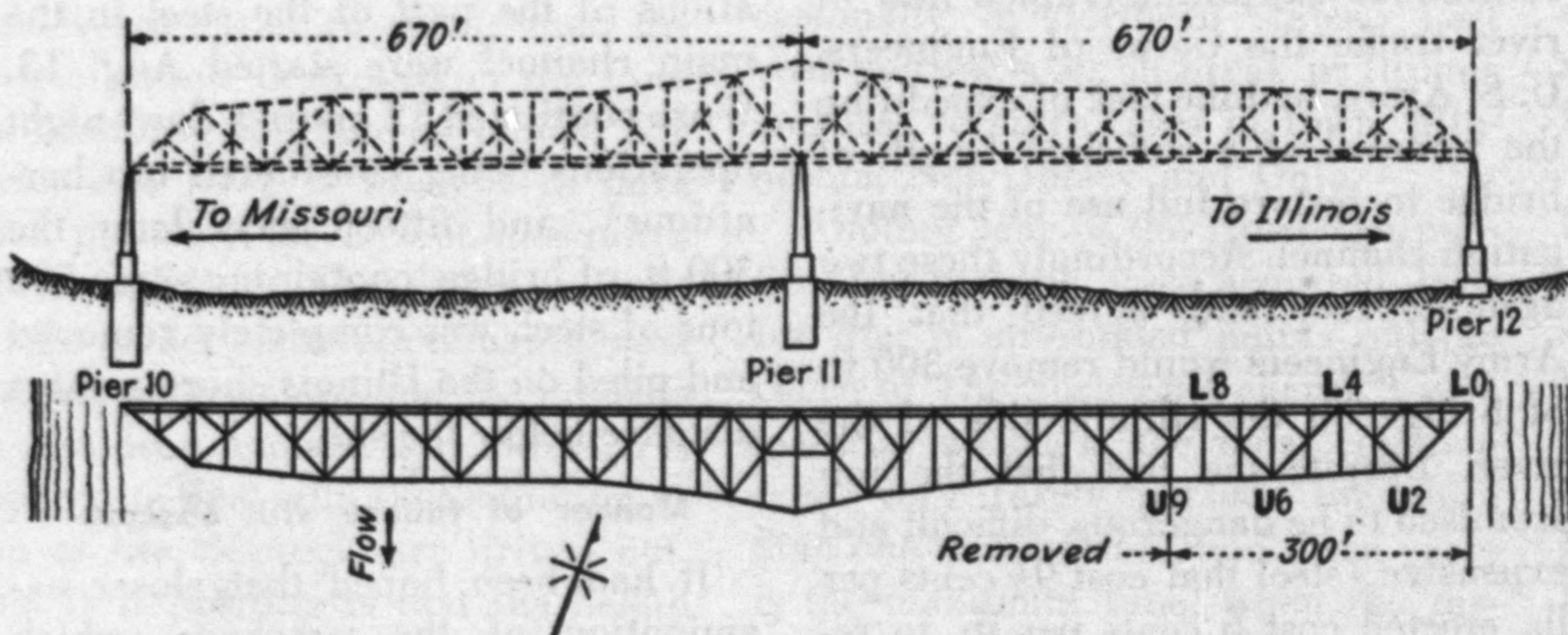
It had been hoped that closer examination of the wreckage, which salvaging afforded, might throw new light on the manner of the failure (*ENR*, Aug. 10, 1944, p. 138), but



After a twisted mass of dynamited steel was raised above the river, members were burned into convenient lengths for loading onto barges.

only a few new facts resulted. Examination shows that the trusses made a one-quarter revolution as they fell on the downstream side of the piers. They came to rest with the downstream trusses on the river bottom and the upstream trusses more or less directly above, parts near the center of the Illinois span being out of the water. Very little of the $4\frac{1}{2}$ -in. concrete-filled floor grating was found. It had been lightly welded to the stringers and thus could have broken free and settled to the bottom of the river.

The theory has been advanced that there was a combination of longitudinal movement and uplift, together with the overturning of the two spans. The condition of the sleeve connections, through which the handrails on the approach and on the collapsed trusses were joined, points to a longitudinal movement; although the sleeves were virtually box sections, the truss handrails on the Illinois side of the river pulled out without bending or damaging either the sleeves or the adjacent railing members. It was reported that a similar pull-out action



Salvage operations started at L0 and continued toward the center of span to provide a clear navigation channel.

occurred on the other side of the river.

Also in support of the theory of longitudinal movement, as well as vertical uplift prior to overturning, is the condition of the cast steel road expansion devices remaining on adjoining spans. These are of the conventional finger or tooth type, teeth about 18 in. long. They have been installed in four sections across the roadway. On the Missouri side of the river, the three downstream sections remained intact, while the first section was torn off as the span turned over. At the river pier on the Illinois side, the downstream section of expansion guard remained in place although it is bent slightly upward. The other three sections tore off shearing the $\frac{3}{4}$ -in. bolts that had anchored them to the approach road slab.

While the condition of the adjoining handrails and roadway expansion devices suggest that some longitudinal movement occurred at each end of a two-span continuous truss at the time of failure, both movements could have occurred simultaneously. Furthermore there is no evidence to indicate that the truss, as a whole, moved longitudinally. It has been suggested that one end of the span could have pulled away from its connecting member followed by a similar movement at the other end.

Other facts developed by recent examination show that virtually no damage occurred to the top of the pier on the Illinois side, while the west pier showed small amounts of spalling and abrasion. Considerable spalling occurred at the center of the largest of which was a piece roughly 12 in. across and 6 in. deep, probably gouged out when the stream top section of the fixed shoe hit it with terrific force. The lower sections of the center pier shoes are in place and practically undamaged. The upstream pin lies on top of the pier and shows signs of the Lomas having been stripped off. As to the rocker bearings at the expansion ends of the spans, three of them went with the trusses. The one at the downstream end of the east pier lies on the bearing plates, although skewed about 45 deg. out of position in being wrenched from the sole plate on the bottom chord of the truss. 16 connecting bolts were snapped clean.

Most of these facts, it will be

to evidence visible above the water. Actually, the principal new information supplied from the salvaging is that the bridge floor broke away from the rest of the structure

Salvage equipment

As would be expected, heavy equipment was needed to dislodge the tremendous mass of steel from the riverbed, bring it to the surface, after it had been dynamited into removable sections. To insure ample lifting equipment and to expedite the work, the agreement with the contractor called for the U. S. Engineers to furnish their 25-ton derrick boat, the "Hercules," together with its 5-man crew.

The Hercules, whose keel was laid in 1940, has an 85-ft. boom mounted on a 40x112x8-ft. all-welded steel barge. The boom carries a 16-ton upper block and an 8-ton upper block with 1,640 ft. of 1½-in. steel rope forming a 16-part line. Its power is supplied under automatic control and at 125 psi. pressure. The contractor's main equipment included a 25-ton completely revolving derrick, with an 85-ft. boom, mounted on two 26x110x6½ ft. all-welded steel barges; a 1½ cu. yd. clamshell crane with a 65-ft. boom hav-

ing a lifting capacity of 12 tons and securely lashed on the deck of a 26x110x6½ ft. steel barge; and a 50-ton capacity A-frame carrying a 13-part line block and falls operated by a steam hoisting engine and mounted on a steel barge. Both the Hercules and the A-frame lifting barge were well ballasted with water in their rear compartments.

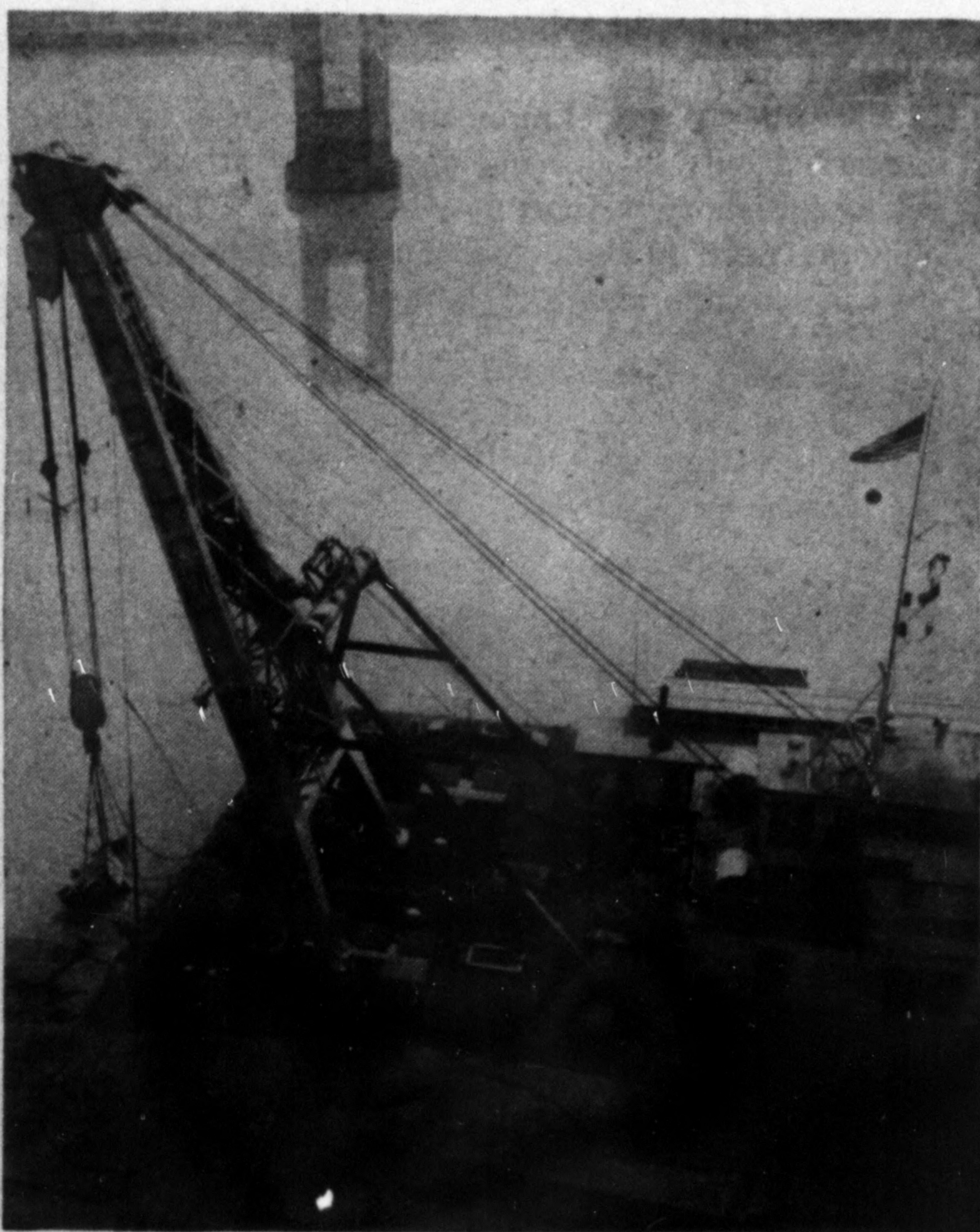
Other equipment on the job included a twin compressor to furnish air to the divers, a 2-way communications line from diver to tender, six 15x40x4-ft. all-welded steel ponton "work flats" and a 250-hp. diesel tow boat for towing salvaged steel to the river bank.

To handle all of this equipment, the contractor used the following crew: 2 construction superintendents, 1 chief engineer, 2 iron-worker foremen, 10 iron workers, 4 power equipment operators, 3 boat operators, 2 deck hands, 3 oilers and firemen, 2 divers, 2 diver-attendants, 1 powder "monkey" and 4 laborers. Although 2 divers were constantly on the job, only one worked at a time. They carried 212 lb. of equipment and leadweights to help steady themselves in the river current.

First operation in removing steel

was to anchor the Hercules immediately upstream from the second panel point (L2) of the bottom chord. The 25-ton derrick was anchored in a similar position on the downstream side of the hip joint (U2) of the top chord. With the barges in these positions, a lift was made at L2 and U2, with the hope that the end of one or both trusses could be raised above the water sufficiently to permit burning into sections convenient for removal. But this plan failed. So great was the weight of end rockers and the steel members, framework and floor system in the two 33½-ft. end panels that the truss span bent sharply at the pickup points and no steel could be raised clear of the water.

Slings were released, the trusses settled back into position and the two end panels of the upstream truss were blasted off under water at the original pickup points. While this work was going on, the two other derricks removed all steel visible above water, or any that could be lifted sufficiently to permit burning into lengths short enough to handle. This included mainly the upstream truss between panel point 6 and panel point 9, which marked the end of the salvage contract. The next operation was to re-



Salvaged steel was loaded on barges by a revolving crane and a crawler crane (left) while the U. S. Army Engineer derrick (right) moved out over the bottom truss chords to do the heavy lifting.



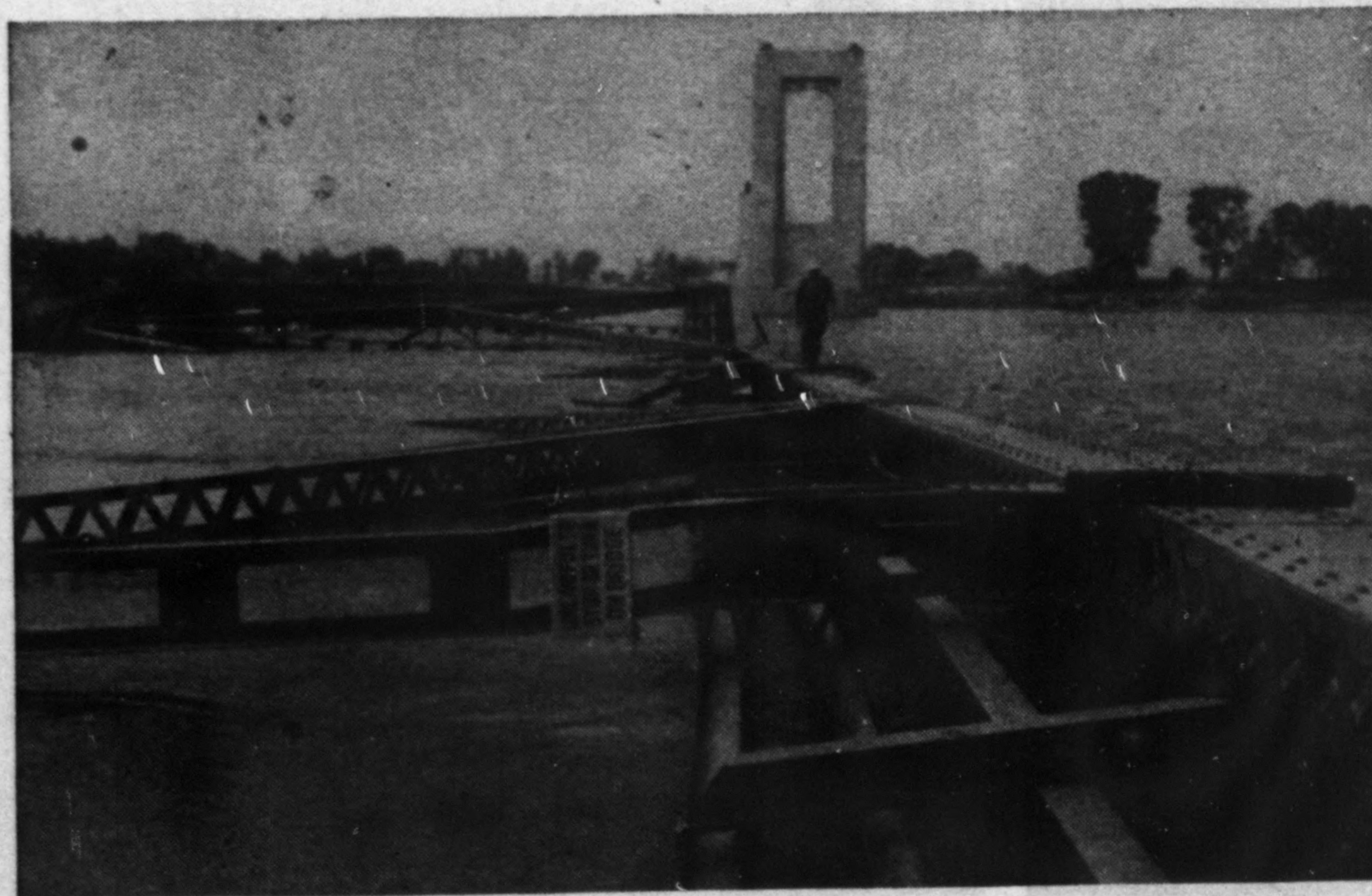
Some idea of the hazards of the work is shown in this action picture of workmen attaching slings preparatory to burning sections for removal.

move all of the upstream truss and as much of the cross-bracing and floor system as possible between panel points 2 and 6.

Since the Army Engineers were concerned only in clearing the channel in the shortest time possible, no attempts were made to salvage any steel in reusable lengths. There were,

however, several sections of cords and web members removed in straight lengths up to 30 ft. Whether such members have a salvage value for reuse in other structures is problematical.

The really tough job was to remove the downstream truss, some of which was embedded in sand, particularly



One of the first salvage operations was to cut and remove all steel projecting above the river.

towards the end of the job. The sages that the longer the removal, 1,040 ft. of bridge stays in the the more difficult will be its removal.

Removal of the downstream progressed from the Illinois toward the center of the first span. Hercules worked along the chord, which lay upstream, and most of the heavy lifting. The hoist-barge operated directly on the Hercules and along the stream side of the upper chord. The chief purpose was to snub the masses of steel above waterline. The different members were cut into convenient lengths.

The 25-ton derrick barge followed along close behind the other barges to pick up members clear of the wreckage and place on a towing barge, moored nearby. The fourth barge, equipped with a 12-ton capacity crawler crane, proceeded immediately in front of the frame barge. It also loaded the wreckage onto a towing barge.

No burning under water

All steel under water was kept apart. Burning under water was out of the question because of the visibility in the muddy water and because of the danger to workers burning apart steel members under strain. Even above water operators of the oxygen-acetylene torches had to exercise great care to avoid serious injury from the predictable movement of a steel member burned in two while under severe torque or strain.

In some instances such members could be lashed securely to prevent movement. But in others the use of a snubbing line or cable was not permitted of their use. In these instances the torch operator was careful to stand clear of the expected movement of the member to be cut. Even then he was always tied to a safety line running to a barge in the event the springing of a steel truss member toppled into the water. So carefully was the cutting of steel planned and executed that no such accidents occurred.

Blasted under water at

After a few trial blasting operations, it was found best to blast the trusses at their panel points. The general procedure was to send

own to place dynamite charges on each chord member, vertical and diagonal meeting at the joint.

In general, it was not necessary to cut the floorbeams loose and usually it was possible to lift them out while attached to the joint of the truss. Sometimes this took considerable tracking and pulling to break the rivets in the connections at one end of the beam.

In the early stages of the job the stringers were not cut loose from floorbeams until they had been hoisted above water. Later, however, it was found the removal operations could be expedited considerably by blasting stringer connections free.

Seldom were the dynamited breaks clean at any joint, but the heavy dynamite charges demolished it sufficiently to permit the tangled steel to be hoisted above water where the different members could be burned off and loaded onto the barges.

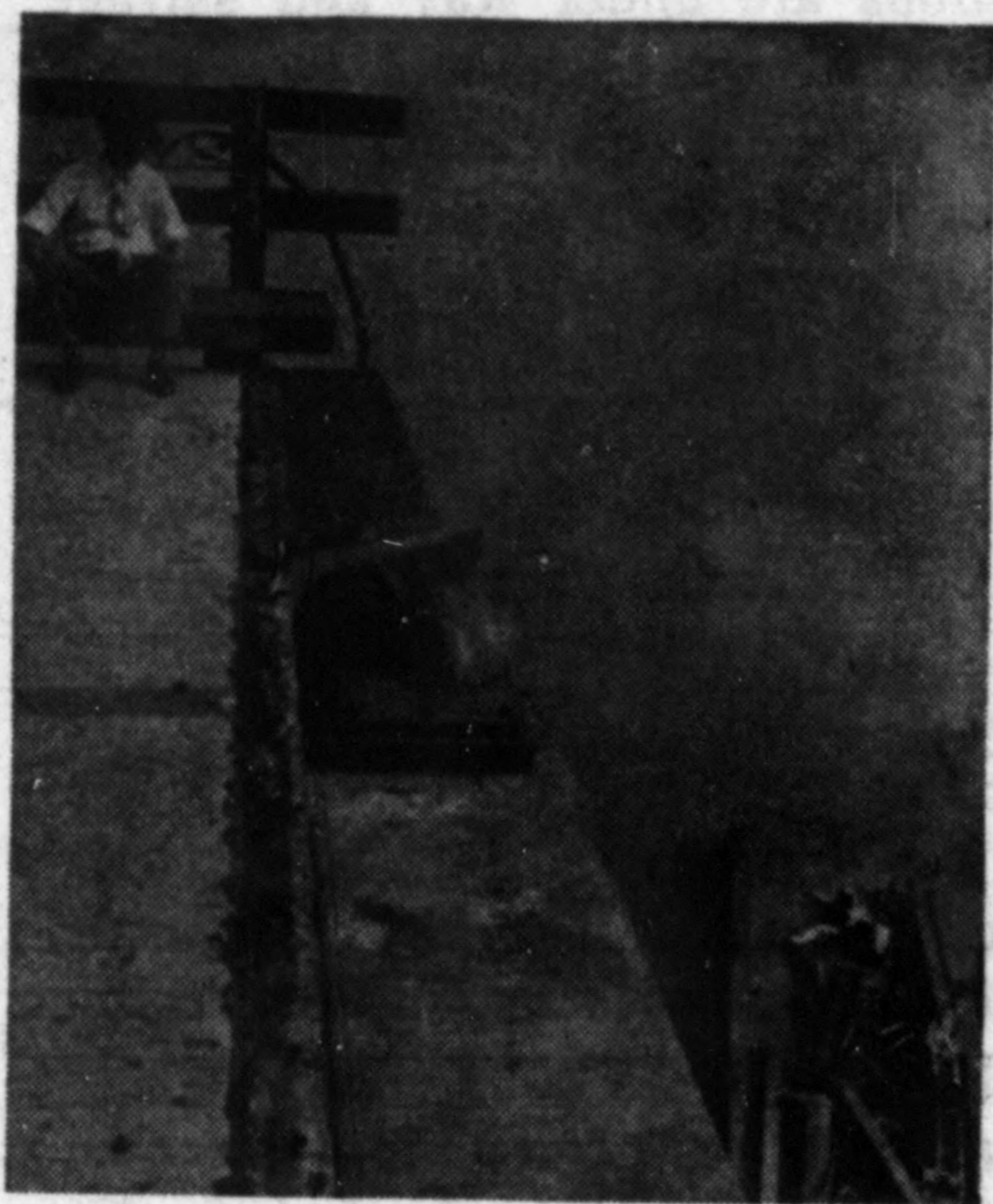
Placing the heavy dynamite charges under water was a dangerous and filthy job. Although the river current did not exceed 5 ft. per sec. it set up heavy vibrations in the steel and dynamite charges had to be placed in total darkness due to the turbidity of the water.

High-velocity gelatin dynamite

In blasting the main members apart dynamite charges made up into cartridges 5 in. in dia. and 16 in. long containing 80 percent high-velocity gelatin dynamite were placed directly against the members and just inside of the gusset plates.

Five dynamite charges were needed near the heavy top chord sections. The heaviest of which was made up of a $28 \times \frac{1}{8}$ -in. top coverplate, two $\frac{1}{2}$ -in. side plates and four $6 \times 4 \times \frac{5}{8}$ -in. angles. Three charges of dynamite were placed inside of the chord section—one against the top coverplate, two against the side plate and one lying on the riverbed. The other two charges were placed on top of the other side plate and angles.

The bottom chord was open, laced on both flanges, and its heaviest section was made up of four $7 \times 4 \times \frac{1}{2}$ -in. angles, two $12 \times \frac{1}{2}$ -in. and two 26×1 -in. plates. Only four of the 5-in. dia., 16-in. long, 80 percent high-velocity gelatin dynamite charges were required to blast the bottom chords in two. Two of the charges were placed between the chord members and on



As wrecked steel is loaded on barge (lower right) examination of pier top shows handrail expansion sleeves were not damaged, while some of the roadway expansion device is intact. Also, through the shearing of 16 bolts connecting it to the bottom chord, one expansion rocker remained on top.

top of the one lying on the riverbed, and the other two were placed on top of the other side plates and angles. Dynamite charges were held in position with sandbags.



A close-up of a truss joint shows the effect of marine dynamiting.

Only one dynamite cartridge of the same size as those used on chord members was necessary to demolish the verticals and web members which usually were two channels, laced. A 60 percent high-velocity gelatin dynamite was used to blast these sections apart, the heaviest of which were 18-in. channels weighing 42.7 lb. per ft. The dynamite charge was placed between the two channels and held in place with a sandbag. A similar blasting charge was used on the stringers and other chord members made up of plates and angles.

All chord members were silicon steel. Web members, stringers and floorbeams were carbon steel.

At some of the panel points 5 members were torn apart at the same time, using 13 dynamite charges. All were fired simultaneously with a "primer cord" fuse, which in turn was fired with an ordinary electric blasting cap connected to a long insulated duplex cord running to a hand-operated blasting machine.

Attaching the $1 \frac{3}{4}$ -in. steel wire rope sling from the hoisting cable around sections of steel to be removed from the murky river was a tedious and

laborious job. Working in total darkness, usually using one hand, the diver would guide the sling down on one side of the steel to be removed. With the sling hanging vertically beside the steel to be removed, a $\frac{3}{4}$ -in. rope was passed under the steel and up to a barge located on the other side of the member. A firm pull on the rope bent the sling under the steel and up on the other side of the piece.

The rope was then passed over the steel to a barge located on the other side of the member, and pulled tight to completely encircle the piece of steel. With the loop thus completed a shackle attached to the free end of the sling was threaded around the lifting cable and closed with a large cotter key which was locked with a bolt that operated as a cotter pin.

Removal of the remainder of the 670-ft. span on the Illinois side of the river will be executed by the St. Louis Engineer District, U. S. Army. Nego-

tiations are under way and salvage operations will be resumed soon. This will restore the full use of one clear span for navigation. Removal of the other span can then be accomplished by the Chester Toll Bridge Commission whenever it is able to secure the necessary funds.

Col. L. B. Feagin, district engineer, St. Louis District, U. S. Army Engineers, assumed direct charge of removing the 300 ft. of collapsed bridge on the Illinois side of the river. He issued all navigation orders past the bridge site and acted in cooperation with Col. Malcolm Elliott, division engineer, Upper Mississippi River Division, U. S. Army Engineers. Enforcement of navigation orders was by the U. S. Coast Guard, under Capt. Beckwith Jordan, assistant district coast guard officer.

Other employees of the U. S. Army Engineers connected with salvaging operations included: A. A. McFadden,

engineer in charge of service base, and river and harbor projects; Capt. J. A. Thieson, of the derrick barge, the "Hercules"; Ruddell J. Spring and Harry Grieshaber, civilian tow boat pilots, and W. W. Hitt, inspector on river construction.

The contract for removing the 300 ft. of structure, estimated to cost \$50,000, was held by the Massman Construction Co., Kansas City, Mo., constructors of the original bridge. (*ENR*, Mar. 19, 1942, vol. p. 446). The contractor's supervisors on the job included: W. H. O'Donley, general superintendent; W. C. Koontz, construction superintendent; and John N. Newell, chief engineer.

Sverdrup and Parcel, consulting engineers who made the original design of the bridge were represented on the job by R. D. Bane, field engineer.

C. W. Scott is manager and John File is acting chairman of the Chester Toll Bridge Commission.

Large Spud Fabricated of Timber

Massive size combined with strength has been achieved in the huge spud of laminated wood manufactured for a dredge used on the Columbia River by the General Construction Co. of Seattle. The spud is 85 ft. long, 30x30 in. in cross section and weighs, dressed to net size, about 19,200 lb.

In fabrication, 10,345 fbm. of kiln

dried lumber and 320 lb. of urea cold-setting resin glue were used. The laminations are 2-in. rough stock dressed to $1\frac{1}{2}$ in. net, of random width.

The pieces in the spud were laid out one lamination at a time and one upon the other. All edges were coated with fresh glue. Pressure was applied by means of clamps both vertically

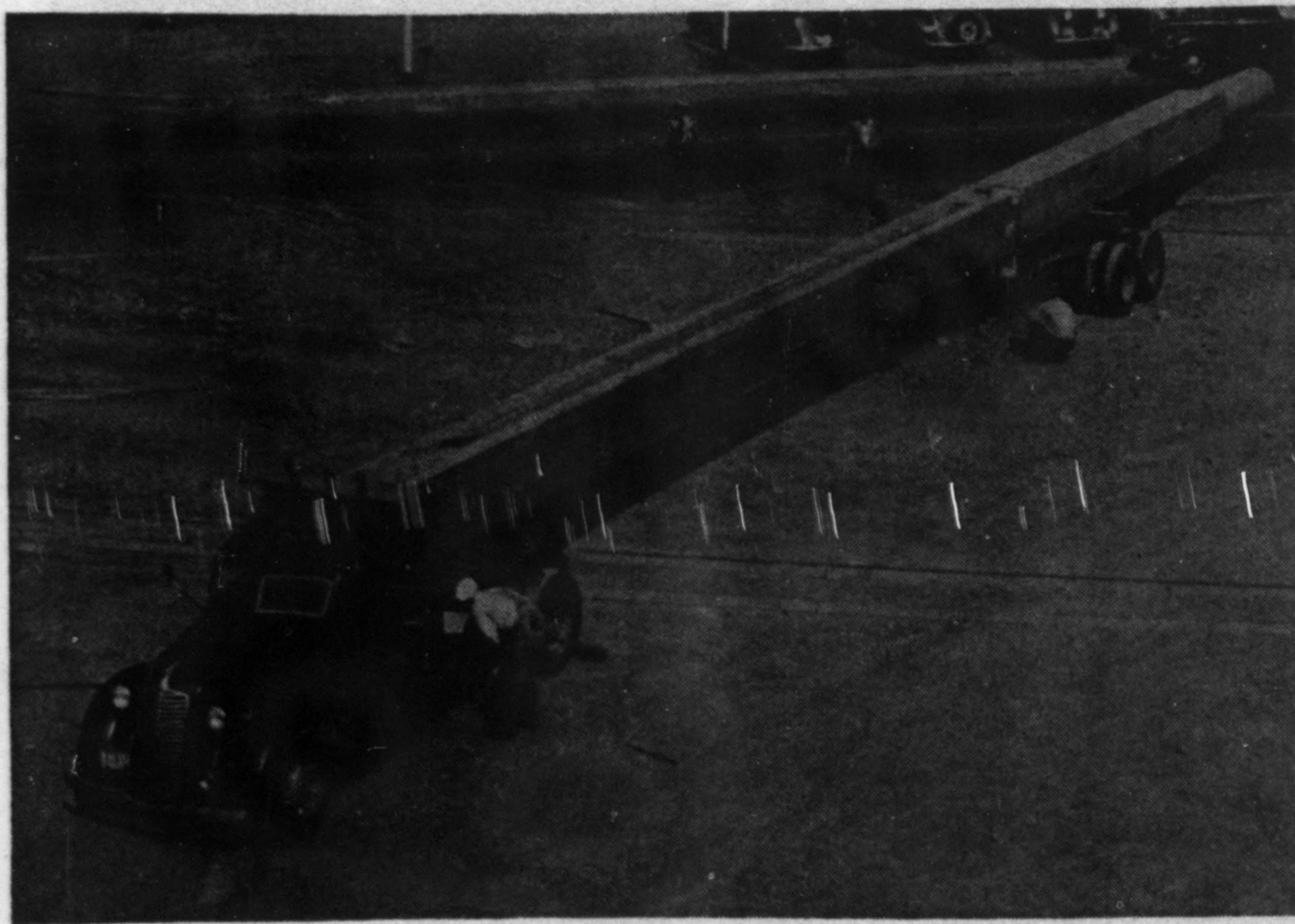
and horizontally, and was retained until sufficient time had elapsed for the glue to set properly. No nails were used in the timber.

At one end the timber was rounded off by hand tools to fit it inside a steel spud point necessary to protect the bottom end of the stick in contact with the river bottom. A long vertical groove, starting at the top of the timber and running down one side about 36 ft. was tooled to the proper width and depth to take a heavy railroad rail that was used as a sliding guide for the timber while in use on the dredge.

When the clamps that held the timber as it was being laid up were removed, the timber was moved to the yard by means of two lumber carriers, one near each end. It was placed on a truck and trailer with two lumber stackers and then hauled to the water front.

The truck was backed down to the water's edge and bindings were cut loose, allowing the timber to slide down the rollers on the truck into the river, where it was towed to the site of the dredge. It was placed in the spud channel of the dredge by means of a block and tackle applied at one end, which raised it directly out of the water and stood it on end in its channel.

Timber Structures, Inc., Portland Ore., was the fabricator.



This 19,200-lb., 85-ft. long timber spud fabricated for use on a Columbia River dredge was transported overland by truck.