fantry Used to Raise Bridge Approaches

ents in Brief—In changing the level of a long highway bridge that is the Tennessee River valley in the area to be flooded by Kentucky a high gantry crane was used to raise sections of the approachets. The crane picked up 90-ft. sections of deck and held them while work was set. Later, after the viaduct supports had been rebuilt by a gabout 25 ft. to the height of the towers, the crane lowered the section of the deck into final position.

pproaches to the Tennessee River way bridge at Eggners Ferry, above pool level of TVA's Ken-Dam commands attention beof the length of the deck raised unit, the method employed to ort the elevated deck sections unew tower bents could be conted, and the procedure followed ilding the new bents.

n air hoist.

Al Coker,

ge. Major

in charge

presenting

Col. Chas.

dquarters.

itted that

emoved in

nger inci-

ce. Since

plete per.

is demon.

of the le ground

uld have

ellant the

k. Any

ne forfei-

and ma-

preciable

ne law is

penalty

a minor

he other

ent.

what be-

stantial

Cordozo

e courts

This

ts face

n Mor-

, 79A,

of sub-

se who

ndeav-

in all

articu-

ORD

m the dam on State Route 68, the sinal crossing was about threeters of a mile long. Its main strucconsisted of six through-truss
as varying in length from 151 to
ft., an east approach was made up
eventeen 45-ft. steel-beam, condeck spans supported on steel
ers, and a west approach consistof 21 similar spans. These apaches were on a grade of 4 percent
to the channel spans, which were
except for the transition in the
spans.

imum pool level and to provide cient clearance for river traffic in main channel, most of the truss I-beam spans had to be raised. The of the piers supporting the ses were strengthened and inseed in height from 11 to 25 ft. In greater portion of the bents supporting the viaduct spans were inseed about 25 ft.

Trusses raised with jacks

wo hydraulic jacks under each floorbeam were used to raise the masses, the piers being extended periodically as jacking continued. This procedure resulted in no special problems, and is similar to the method in raising the vertical lift rail-

road bridge at Danville, Tenn., which was described in Engineering News-Record April 6, 1943, Vol. p. 486. An exception was the use of special auxiliary jacking girders located below the existing floorbeams for raising the 320-ft. and 366-ft. spans.

An electric-powered gantry crane

of 200-tons lifting capacity was chosen to raise the approach spans. The gantry had a height of 80 ft. and a span of 48 ft. 9 in. It consisted of two portal frames, 50 ft. apart, joined by three lines of struts, one near the base, one at midheight and one at the top. An inverted kingpost truss at the top of each portal was pin-connected to the top of each leg and was knee-braced to one leg, an arrangement that avoided indeterminate stresses arising from any inequality in the elevation of the supports. The bottom of each leg was pin connected to a steel skid that was supported on two skid rails set 20 in. apart.

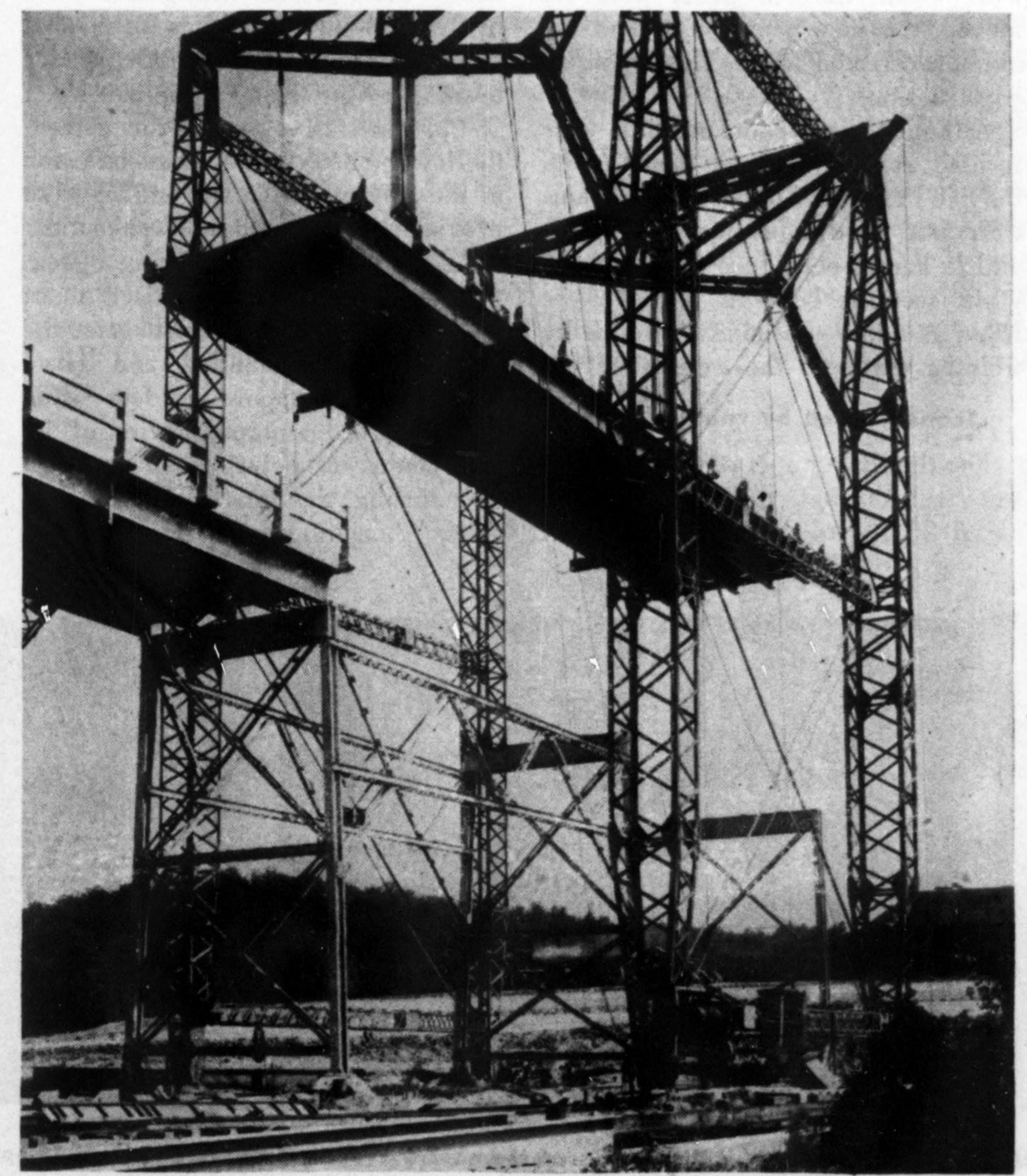


Fig. 1. By means of two 15-part fall lines, the gantry crane raised 90-ft. sections of deck (two spans) to permit supporting bents to be increased in height.

GINEERING NEWS-RECORD • June 1, 1944

Engineers of the Tennessee Valley Authority had found the gantry economical in raising a highway bridge at Guntersville, Ala., (ENR, Dec. 7, 1939, p. 757). On the Guntersville project the deck was raised in onespan sections, swung through 90 deg., and lowered to the ground between the bents. The same procedure was considered for the job herein described, but was considered to be more expensive than raising two spans as a unit and supporting them on falsework while the new concrete bents were constructed.

Old bents utilized

Because of the scarcity of structural steel, maximum reuse of the old steel was necessary. The plan finally decided upon was to place the old tower bents on new reinforced concrete tower bents of sufficient height to bring the deck up to grade. Such a plan meant that the deck sections had to be supported on temporary falsework while the old bents were removed, the new bents built, and the old steel erected on the new concrete construction.

Although the expansion joints were located at every second panel point, the five lines of 27-in. steel floorbeams were not continuous for two spans, and so had to be spliced to permit the 90-ft. sections between joints to be lifted as a unit. Splicing was done by welding plates to the webs.

Section raised by two fall lines

For the lifting operation, two holes were cut through the concrete deck about 20 ft. from each end of each

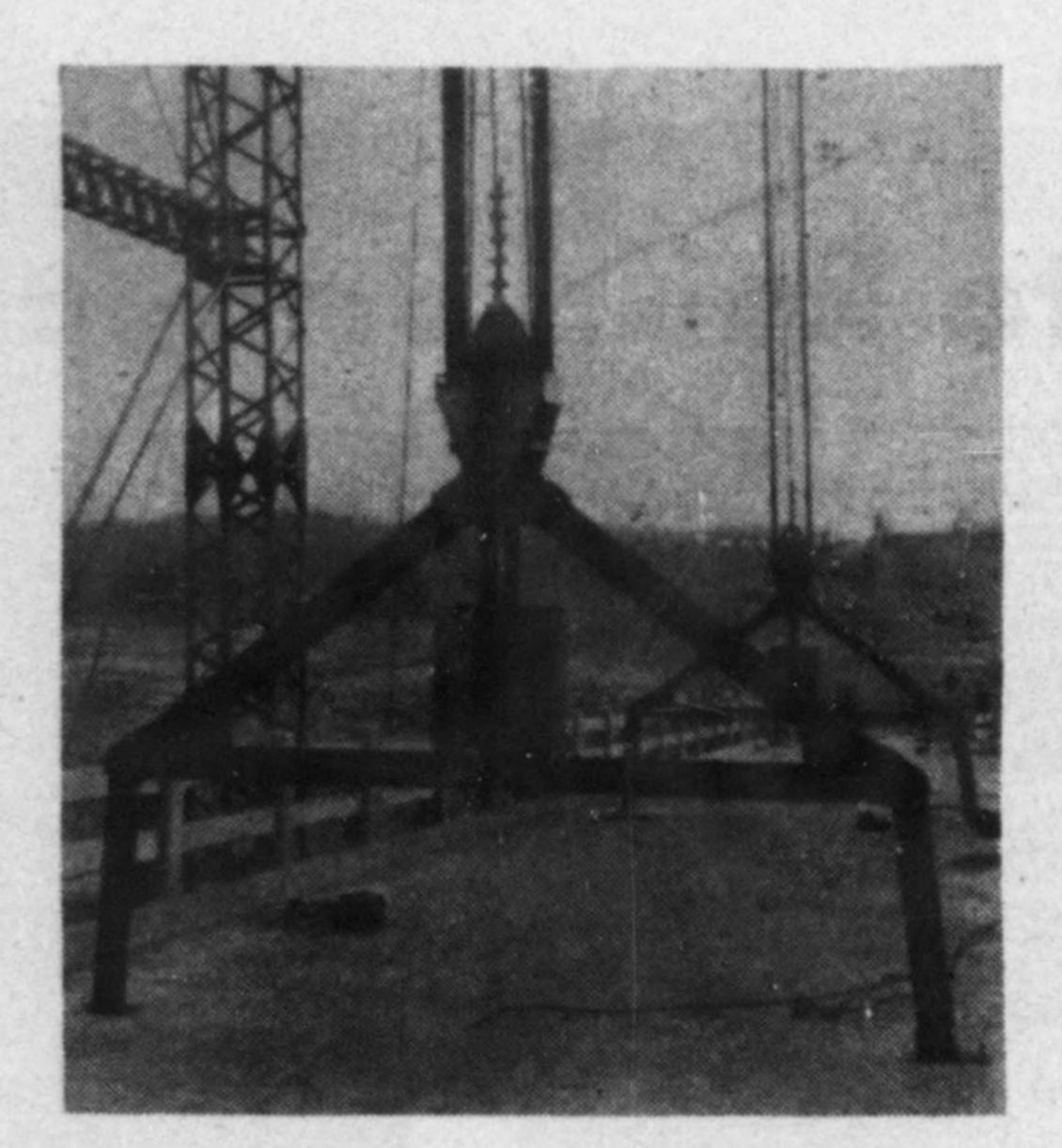


Fig. 2. The hitch to the needle beams below the deck was made through two holes cut in the floor at each beam.

two-span section of the deck to permit the legs of the lifting sling to be attached to needle beams below the roadway (Fig. 1). The needle beams were spaced so that the moment in the splices in the floorbeams was practically zero. Actual lifting was done by two 15-part fall lines suspended from the trussed tops of the gantry towers, power being supplied by a 25-hp. electric motor suspended from the lower horizontal strut at one side of the gantry. Automatic brakes were provided in case the power supply failed while a span was being raised.

After a span had been raised about 30 ft. above the old steel towers, the towers were dismantled and falsework built to support the deck while the reinforced concrete portion of the new bents was being constructed. In dismantling the old steel, only the rivets connecting the horizontal brac-

ing to the columns were cut av which permitted the two columns each bent to be removed as a m thus expediting removal and reasse bly of the old steel.

Two temporary falsework town each consisting of two 2-column st bents was used under each on length of the deck (Fig. 4). The in. steel columns of these bents rest on 5x5-ft. spread footings. The fa columns for a given tower were spaced as not to interfere with a struction of the lower reinforced-or crete section of the new bents.

About an 8-hr. day was requir to raise a section of the deck, remo the old steel, add the temporary fall work, and lower the section on to falsework.

Bents built in three pours

4. Temp

t. section

ports. T

old foun

d by dun

y-down

h a 75-ft

rated in

When the

n section

atry cran

inges of

steel be

t the lov

ded in

is win

ims us

ck and

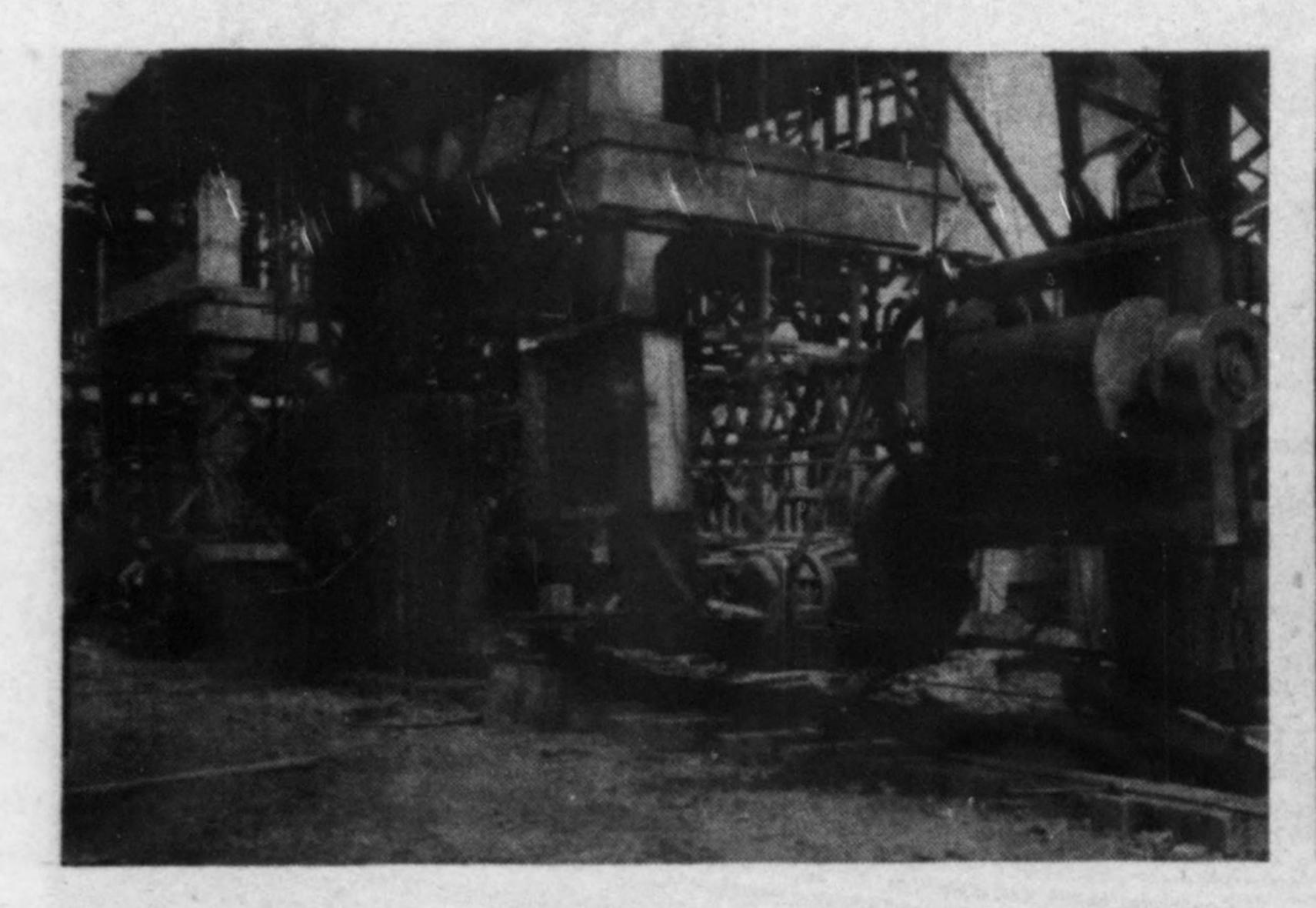
aid o

ethod wa

suppor

The new reinforced-concrete tow bents rest on the footings of the steel towers, the new concrete bei tied to the old foundations as ill trated by Fig. 5. Their tops were a ried to an elevation sufficiently about pool level to protect the steel for wave action. In constructing the inforced-concrete base of a typic tower bent, three pours were use First, the four lower horizontal str were poured, next the four column ary false and finally the four top struts. Ge steel to erally, three days were required to struction the reinforced concrete work at each thod was tower bent, four complete sets of sta forms being available to expedite the part of the construction.

A concrete of 2,500-psi. strength 28 days was used. It was mixed by 1-yd. mixer at a central plant, deli work.



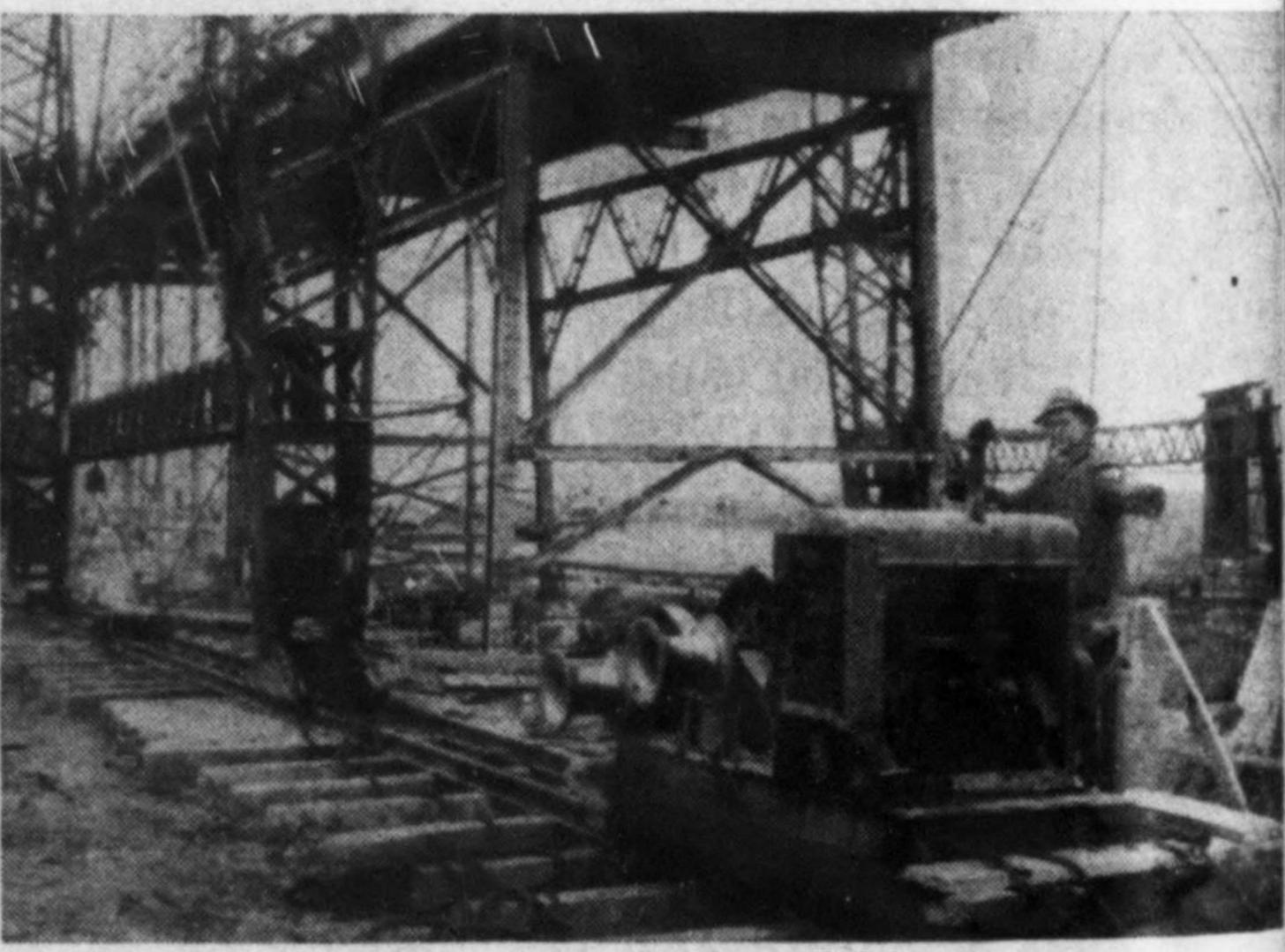
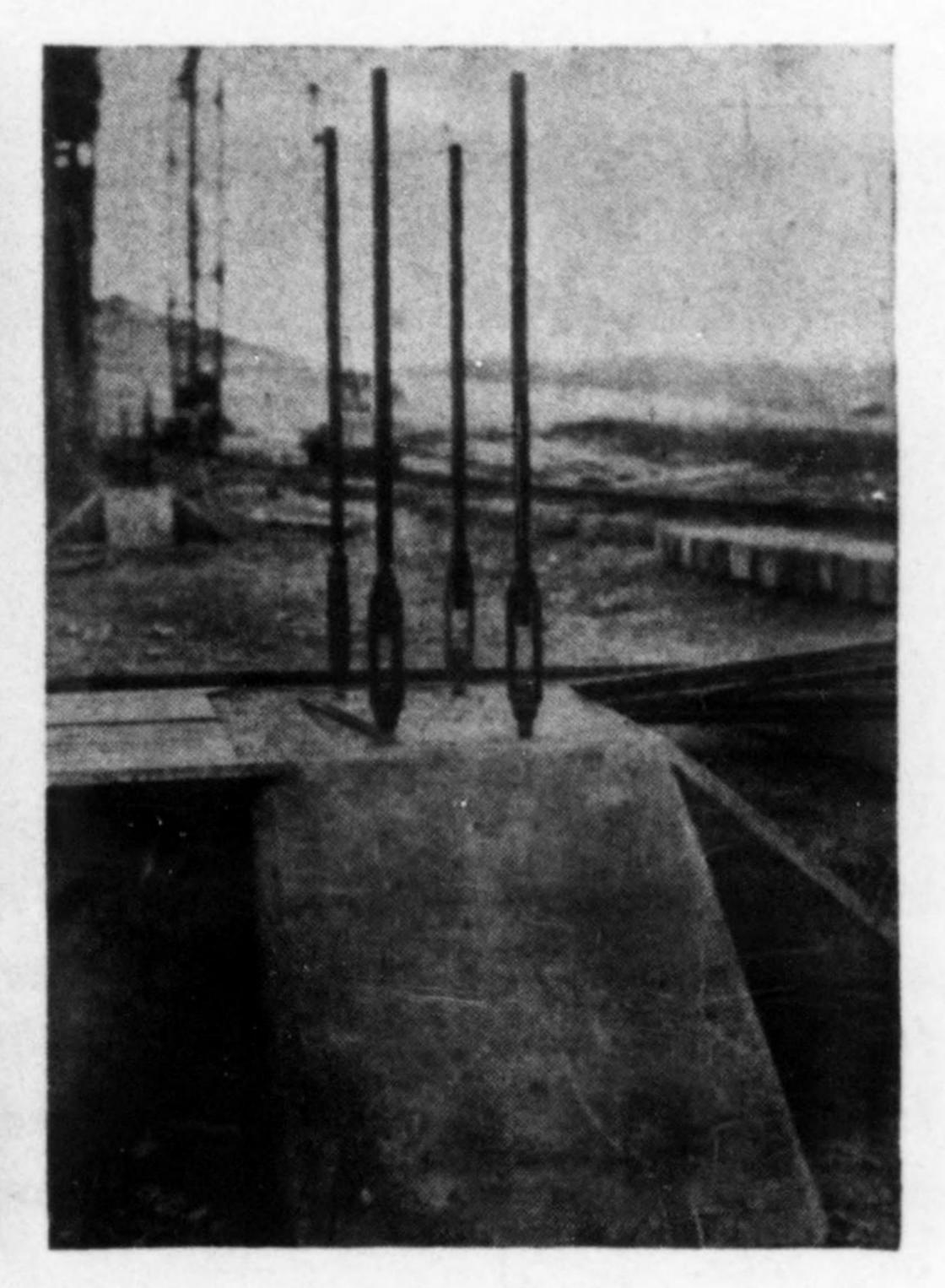


Fig. 3. Power for both lifting blocks of the gantry crane was provided by a 25-hp. electric motor (left) supported of the side between legs of the crane and a short distance above rail level. Power for moving crane forward (right) supplied by a gasoline-powered, double-drum winch that slid along one pair of the crane rails. The winch also did in erecting the needle beams below the bridge deck, removing and re-erecting the old steel.



4. Temporary support for the raised sections of deck (left) was provided by 4-column steel towers, two under each section of the deck. These towers were so arranged as not to interfere with construction of the new concrete ports. The new concrete towers were built on the old tower footings (right), the new construction being tied to old foundations by four steel dowels threaded onto the anchor bolts for the old steel.

by dump truck, and placed with y-down bucket handled by a crane h a 75-ft. boom. All concrete was rated in place.

ags. The fa

wer were

ere with o

einforced-co

bents.

was requir

deck, remo

porary fal

ion on to

e pours

ncrete tow

gs of the

ncrete bei

ons as ill

ops were a

ciently abou

e steel from

mixed by

plant, deli

ted at a

right) w

also aide

ECOM

cting the When the concrete bent for a twoof a typic in section had been completed, the were use try crane backed up to that point zontal strand supported the deck while the temour column ary falsework was removed and the struts. Gest steel tower was erected on the new quired to struction. An exception to this ork at each shod was at those few spans where sets of statinges of grade required that the expedite is steel bents either be shortened or the lower end of the bents be emstrength and ded in the new concrete. The latter thod was followed, as it required

When the steel erection had been ppleted, the gantry lowered the k in place. In sections where the de was changed, steel plates were ed under the floorbeams at the ports to accommodate the minor nges of slope at the bearings. inge of grade also required replacent of some of the handrails to ke the handrail posts vertical, but erwise little work remained after deck sections were lowered in

ower to move the gantry back and th along the bridge was provided a double-drum, gasoline-powered nch mounted on skids (Fig. 3). winch also handled the needle ams used in lifting a section of the ck and set the falsework steel with aid of a small crawler crane.

work were carried out simultaneously, which permitted rapid progress. On the east approach, all work except reconstruction of the handrail was completed in 2½ months. During this time, operations continued 8 hr. per day six days of week. The contractor's crew number 140 men, a large proportion of these men working on the main crossing and the west approach.

All TVA bridge work is under the direction of C. E. Blee as chief engi-

Because of the great length of the neer, George R. Rich, chief design enstructure, the several phases of the gineer, K. C. Roberts, head structural engineer, and Everett Scroggie, senior structural engineer. Field engineering is supervised by E. M. Arnold, principal civil engineer, assisted by W. J. Starr and J. J. Helton.

> The construction work involved in the raising of the Eggners Ferry bridge cost about \$384,000. It was carried out under contract by the Rust Engineering Co., Pittsburgh, represented by E. P. Wilson, general superintendent.

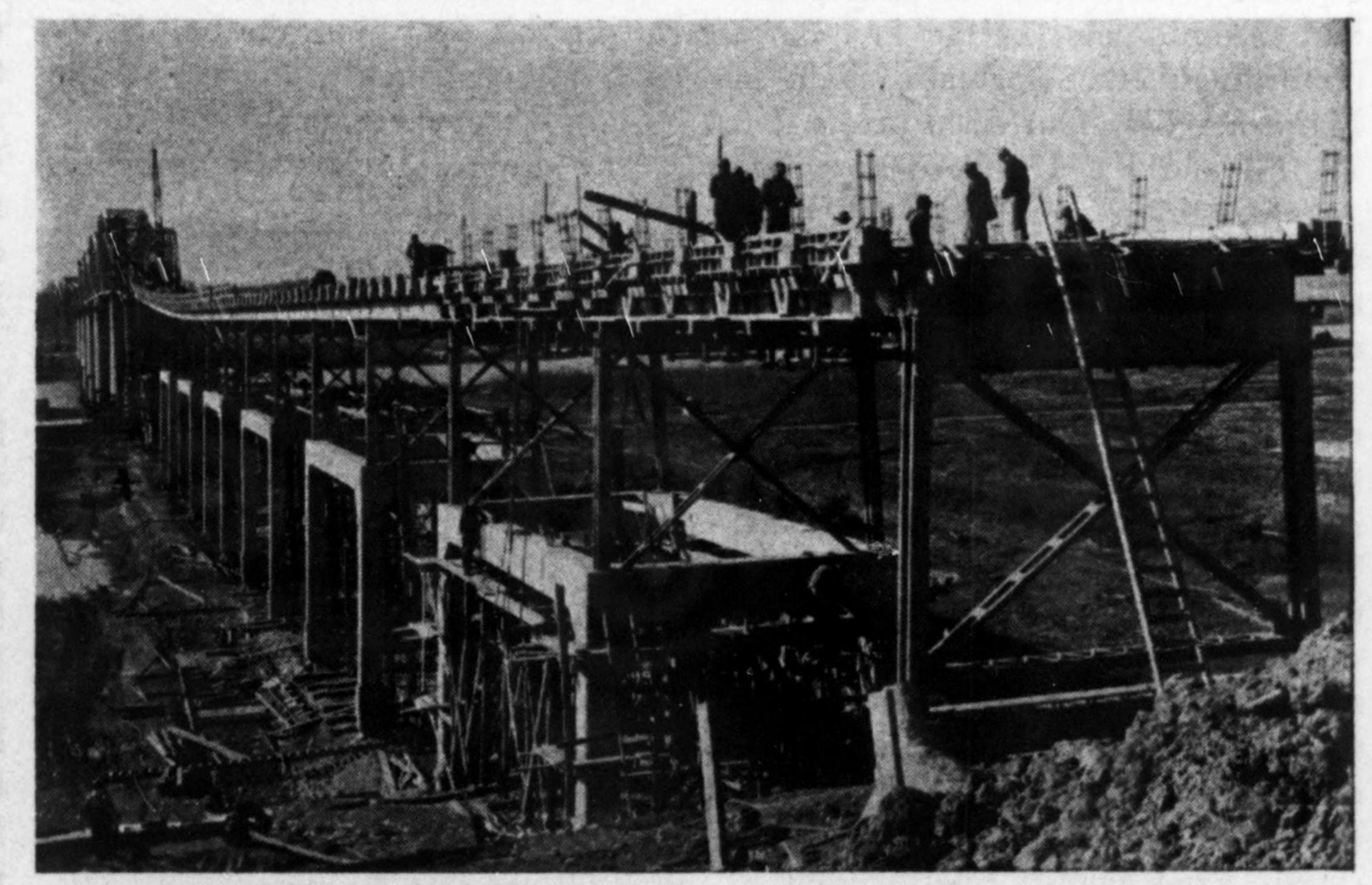


Fig. 5. Work of raising east approach nearing completion, erection of old steel on new concrete towers having been completed. Changes of grade of deck, which required addition of new handrails with vertical posts, were accommodated by embedding the lower portion of the old steel in the concrete.