

Approaches to Historic Bridge Rehabilitation

Case Study #2

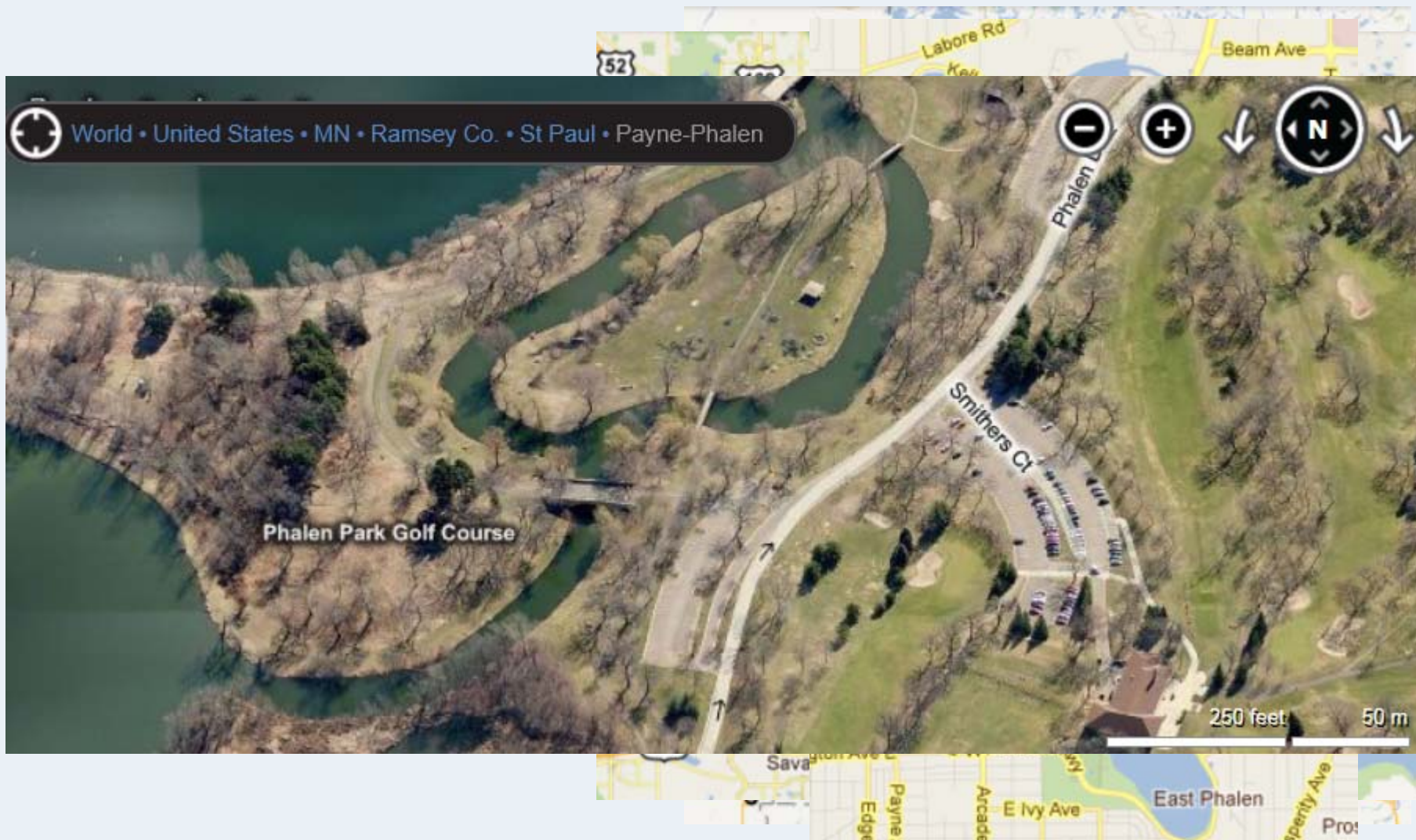
Rehabilitation of the Phalen Park Arch Bridge

Saint Paul, Minnesota

Steve Olson

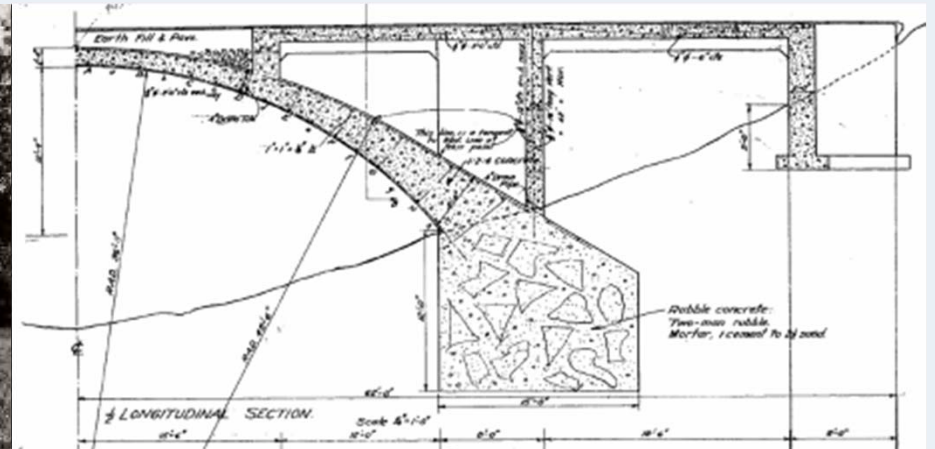


Location and Setting



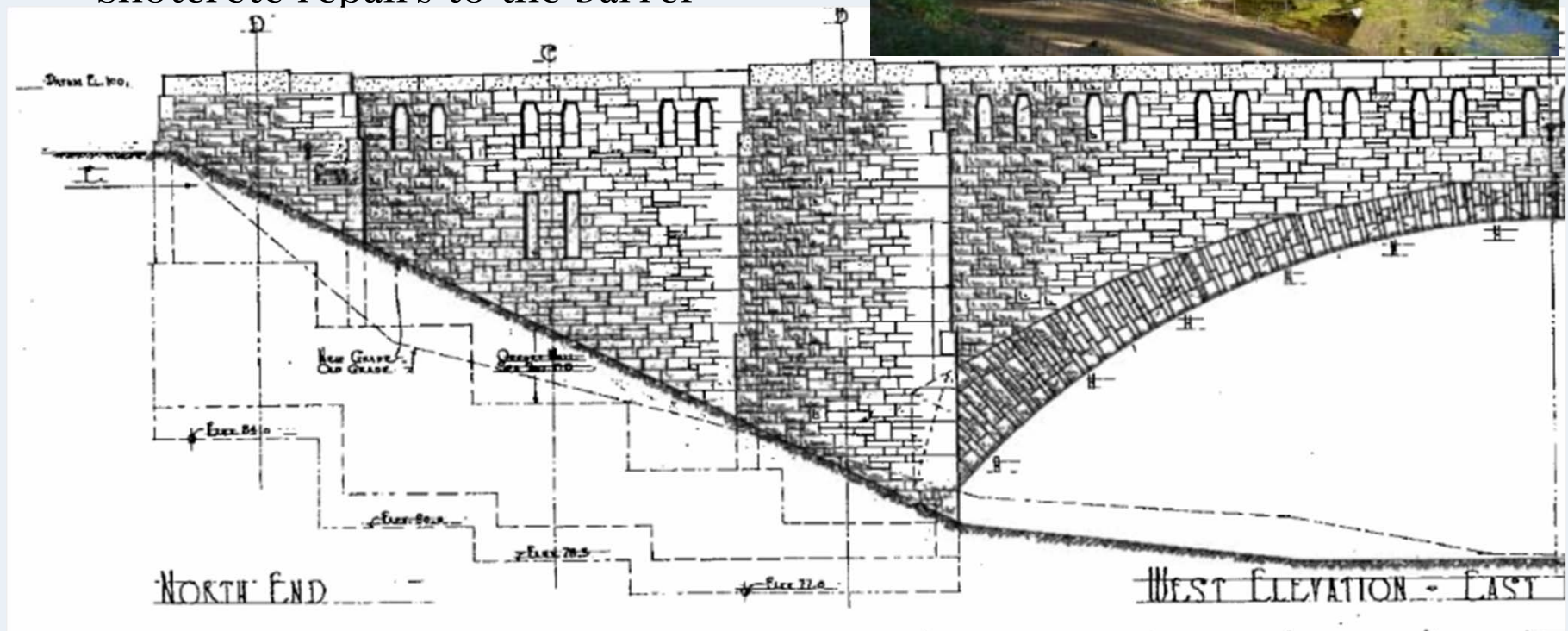
Phalen Park Arch Bridge

- Reinforced Concrete Open Spandrel Barrel Arch
 - Constructed in 1910
 - Overall length 124', out-to-out width 42'
 - 55' arch span, two 18.5' slab spans
 - Designer of Regional Significance
 - C.A.P. Turner – would later design the Mendota Bridge (1926) and several other notable structures in the Midwest



Case Study #2

- 1934 rehab to the “deteriorated” concrete bridge
- Open spandrel walls filled in with stone
- Concrete balustrade railings replaced with stone railings
- Shotcrete repairs to the barrel





Deteriorated components forced the City to close the waterway below the bridge.



Extensive deterioration of the concrete barrel



Due to barrel deterioration and deterioration near each edge, no vehicles were permitted on the bridge



Stone railing was in poor condition. It was too short for current standards and had openings that were too large for current standards.



“Pervious” pavement led to the deterioration of the lower bridge components at concrete joints. A water main sat on the west sidewalk.



Friends of Lake Phalen

"Working to protect and enhance Lake Phalen, Phalen Park and surrounding areas"

- Secured City funding for a rehabilitation study
- “Can you save it? Our sons and daughters were married on this bridge?”
- After the study was complete, they applied for and received a St. Paul HPC “Confidence Award”
- Secured City funding for rehabilitation construction

Rehabilitation Project Details

- *Owner:* City of Saint Paul, Parks and Recreation Department
- *Prime Consultant / Bridge Engineers:* Olson & Nesvold Engineers
- *Architects / Stone Masonry:* MacDonald & Mack
- *Civil Design / Construction Administration:* TKDA
- *Geotech / Material Testing:* Braun Intertec
- *Historians:* Mead & Hunt
- *General Contractor:* Global Specialty Contractors
- *Timeline and cost:*
 - Design 2009-2010
 - Reconstruction 2010-2011 (\$1.3 million) – No federal dollars



Precast Concrete Panel Liner System

Robust structural support system for both the barrel and the spandrel walls



New Arch Ring and Façade Stone

MacDonald & Mack selected a single source that best matched the 1934 stone



Concrete Deck with Approach Panels

Waterproof cap on the top of the bridge to protect lower elements.

Approach panels carry water off the ends of the bridge. Sidewalks removed.



Railing Details

Reused cap stones. Curb integrated into the bottom of the railing for the vertical curve.

Old opening size “too big” to meet standards. Inner stones pinch in to reduce opening size.

Section 106 Issues

- Two periods of significance – original construction in 1910 and rehab in 1934
- Rehabilitation could have been to either period of significance.
- “Friends of Lake Phalen” preferred the 1934 stone version.
- Project historians coordinated the project with the Saint Paul Heritage Preservation Commission.
- Saint Paul Heritage Preservation Commission asked MN State Historic Preservation Office (SHPO) to review.
- SHPO’s only concern was the amount of stone being replaced.
- Once SHPO learned that Bob Mack had recommended replacement of the stone, there were no additional concerns. [Bob is the author of National Park Service Preservation Briefs on masonry]

Section 4(f)

- No federal dollars were used for the project. Consequently, there was no Section 4(f) analysis or environmental document.
- “Can you save it?” is not the same as selecting the most reasonable and prudent alternative.
- The owner did consider a replacement structure.
- A narrower “Conspan” structure would have been significantly less expensive.
- In the end, City bonds were secured to pay for the rehabilitation.

Conclusions

- The benefit of a strong project champion (“Friends of Lake Phalen”) cannot be overstated. They made the project happen.
- A good owner, a strong consultant team, and a good contractor all helped make this project successful.
- A construction sequence video was used as a communication tool throughout the project.
- It’s very satisfying to work on a project embraced by the community. A canoe/kayak parade is planned for next spring’s grand “re-opening” festivities.

Historic Bridge Rehabilitation Strategies – 3 Project Examples

Steve Olson
Principal Bridge Engineer



Strategies

- Laser-scanning
 - Lack of original plans
 - Modifications after original construction
- Integrating new materials
 - Lightweight concrete
 - Self-consolidating concrete
- Structure-in-a-structure techniques
 - Cast-in-place arch
 - Cast-in-place T-beams



Example Projects

- Bridge 82524 (A.K.A., Bridge 5721, or Silverdale, or Manning Avenue). Construction to be complete by July 2011
- Bridge L8560 (C.A.P. Turner arch bridge in Phalen Park). Construction to be complete summer 2011.
- Bridges L8920 and L8921 – Concrete T-beam bridges over the Midtown Greenway – Design is 95% complete



Rehabilitating the Silverdale Bridge (an 1870s Iron Truss)



Introduction to Bridge 82524

- One of 24 Historic Bridges owned by Minnesota to be preserved indefinitely.
- Originally assembled in Sauk Centre in the 1870s. In the 1930s it was dis-assembled and later moved to Koochiching County in northern Minnesota. It carried State Highway 65 over the Little Fork River at this site until 2009
- In 2011 it will begin its third service life





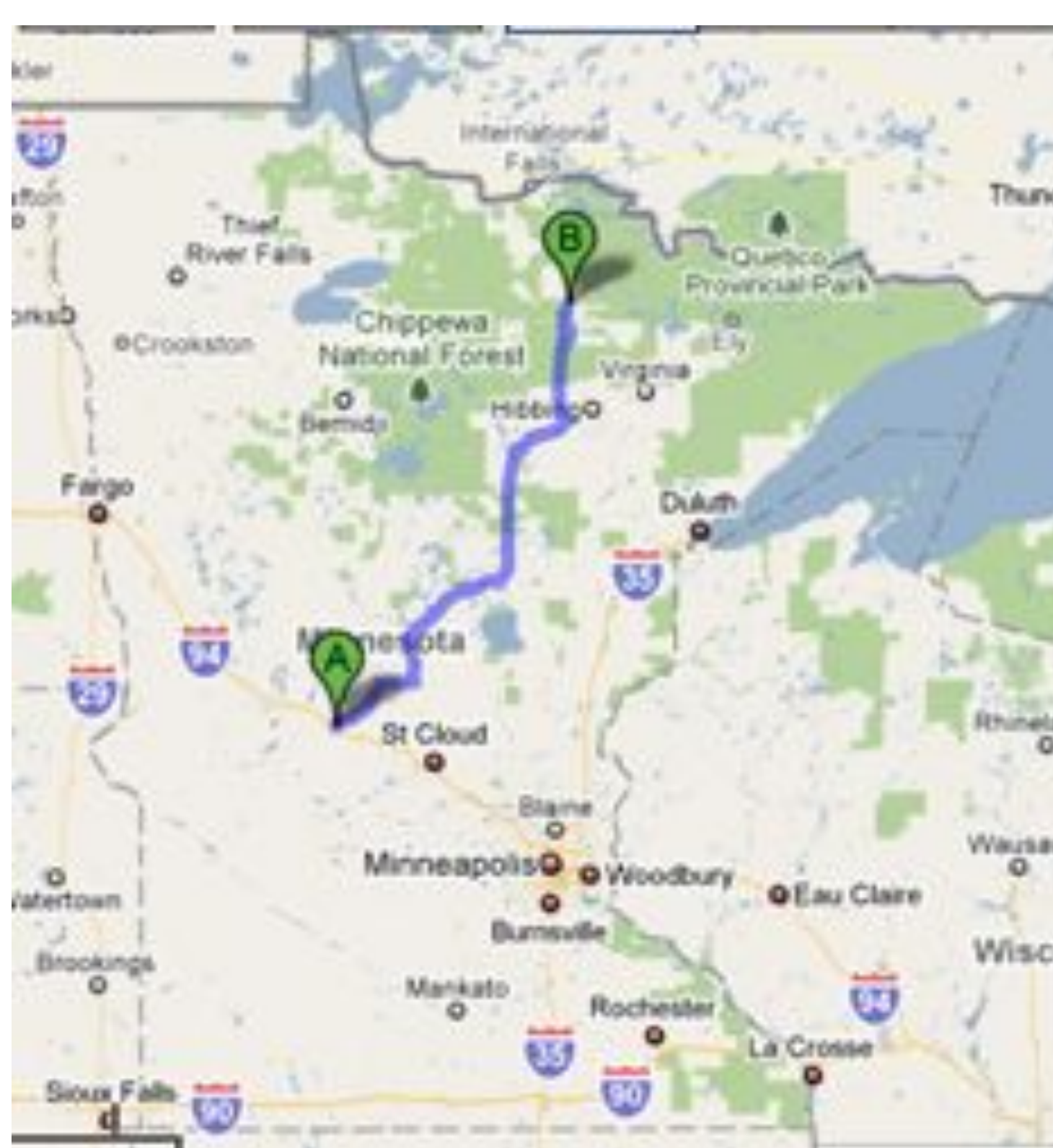
Location 1





Location 1
1870s to
1930s





Location 1 to
Location 2





Location 2
1937 to 2009



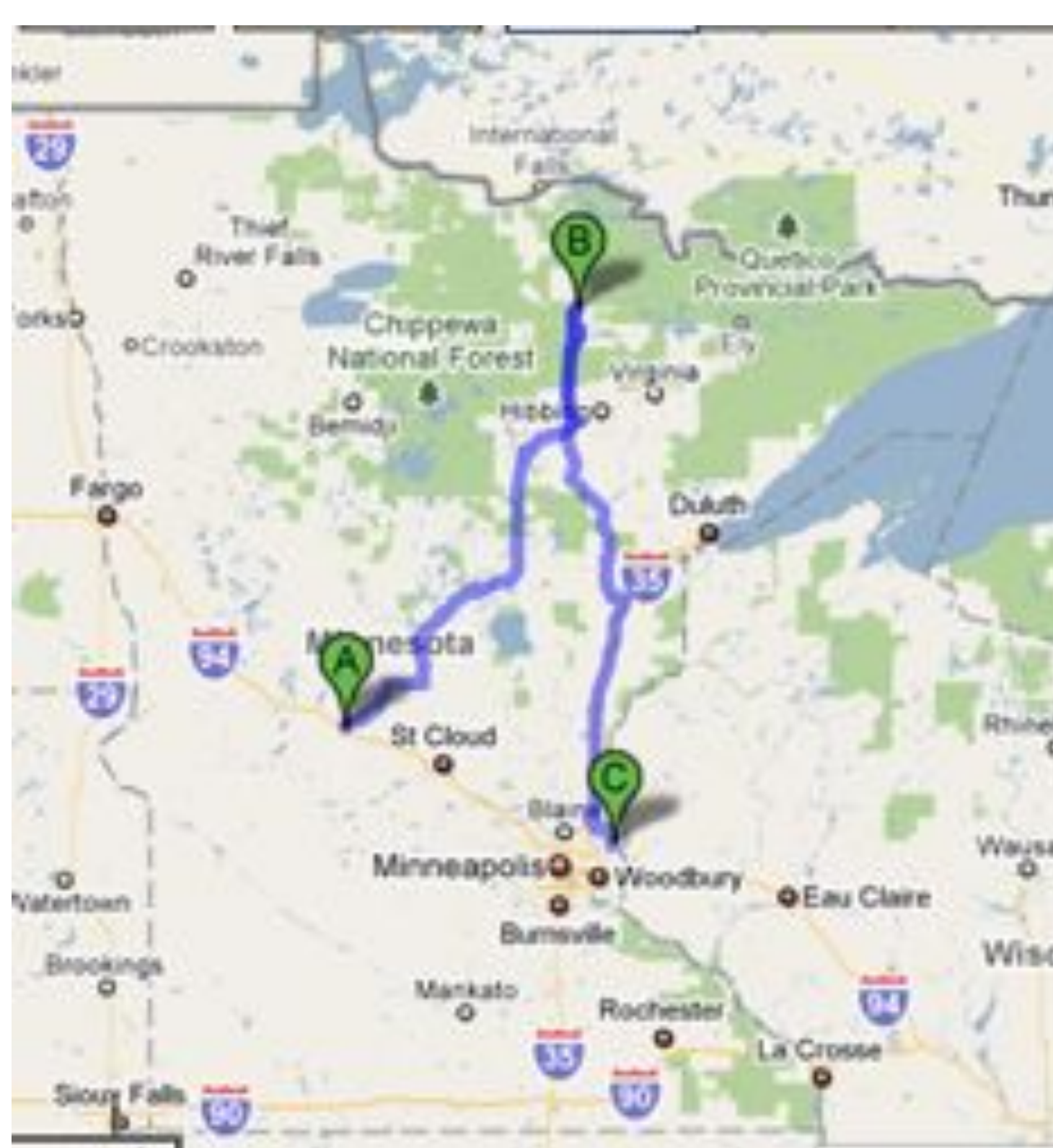


Location 2



Location 2





Location 2 to
Location 3





Location 2 to
Location 3



Location 3



1870s Plans



Laser Scanning Data Collection

1. First data collected on the complete bridge in-place prior to disassembly
2. Second wave of data collected on pieces as they were removed from the bridge during disassembly (used to get fastener patterns etc.)





Silverdale Laser scan Data Used For:

- Replacement Portals
- Replacement End Floor beams
- Evaluation of Damaged Eyebars
- Rehabilitated/Reused Railing



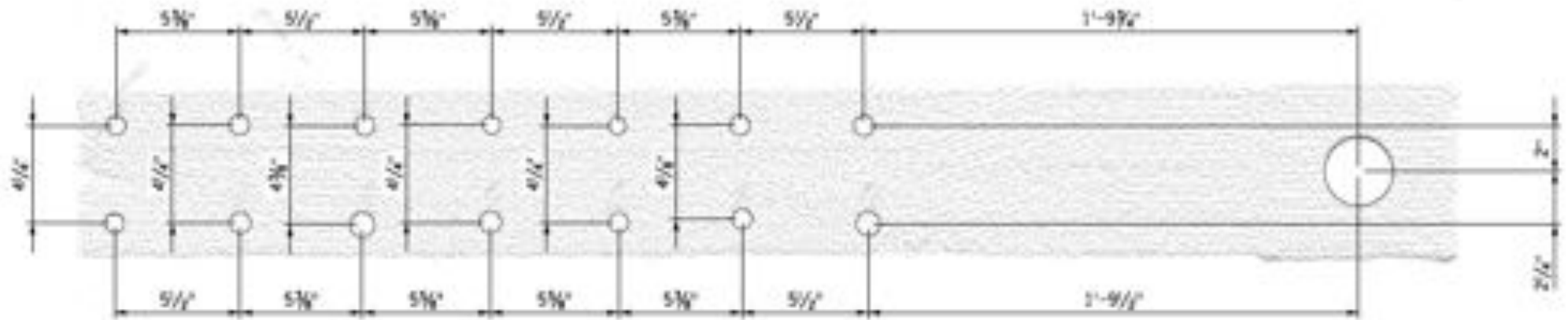








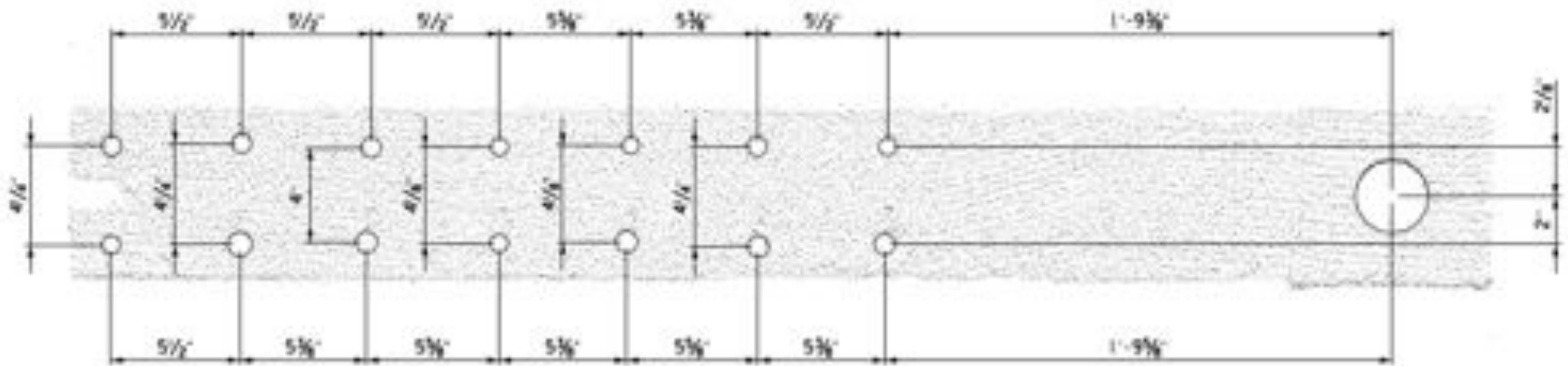
Portal Fastener Pattern 1



DETAIL "1"



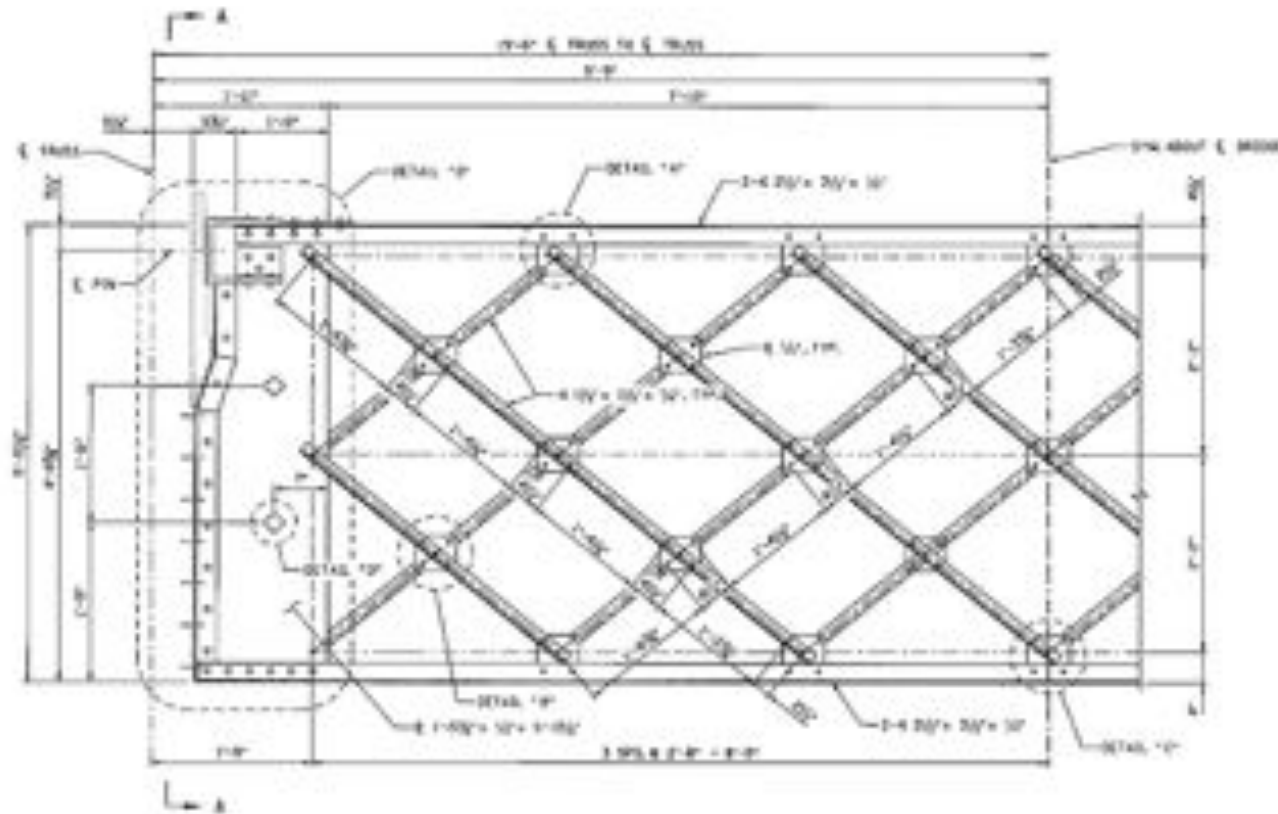
Portal Fastener Pattern 2



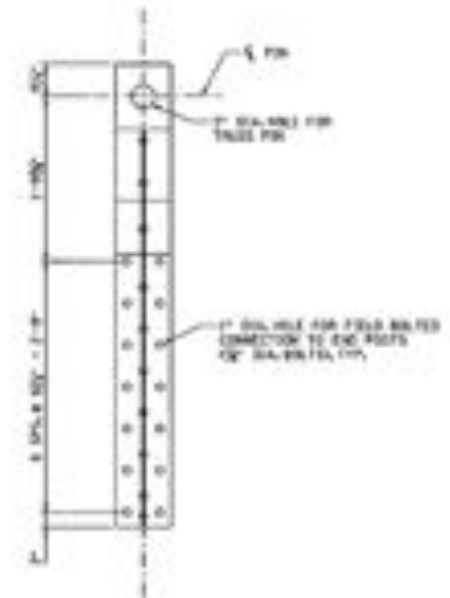
DETAIL "1"



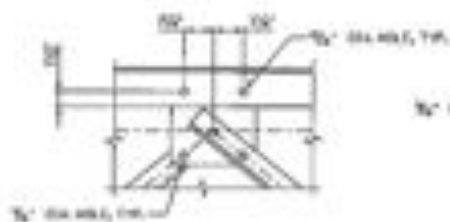
Replacement Portals -1



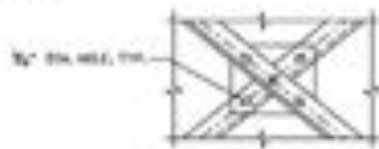
HALF-ELEVATION



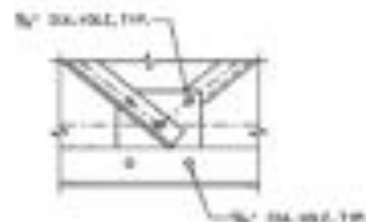
ELEVATION A-A



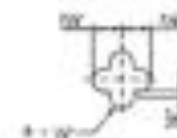
DETAIL "A"



DETAIL "B"



DETAIL "C"

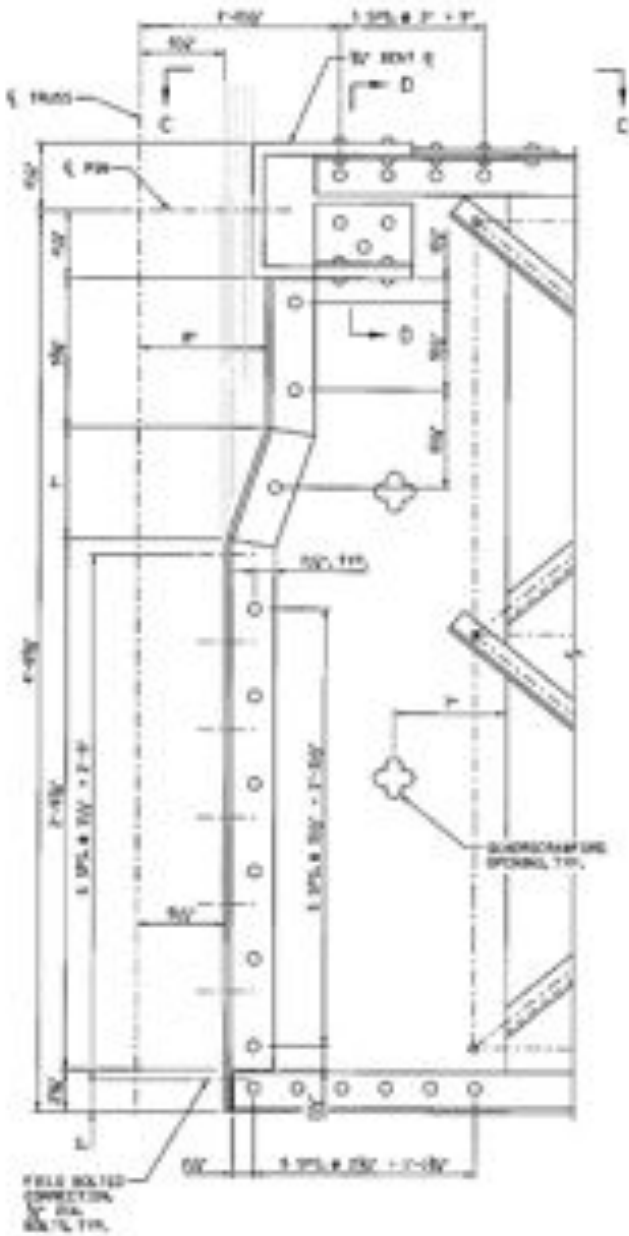


DETAIL "D"
HORIZONTAL SPREAD

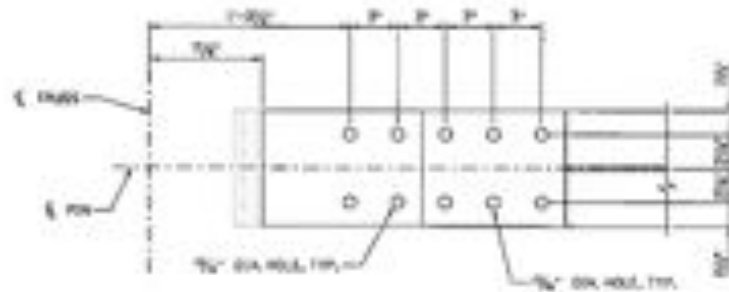
NOTES

- SEE SHEET 24 FOR DETAIL "C".
- ALL PORTAL ASSEMBLY SHALL BE PERFORMED IN A FABRICATION SHOP.
- 1/2" RIVETS SHALL BE INSTALLED IN 3/4" DIAMETER HOLES.
- 3/4" RIVETS SHALL BE INSTALLED IN 1/2" DIAMETER HOLES.
- ALL ANGLES AND PLATE MATERIAL FOR NEW PORTALS TO BE MILLOYD 100K.
- THE PORTALS ARE INCORPORAL TO THE THROUGH ARCHITECTURAL HISTORIC TRUSS.

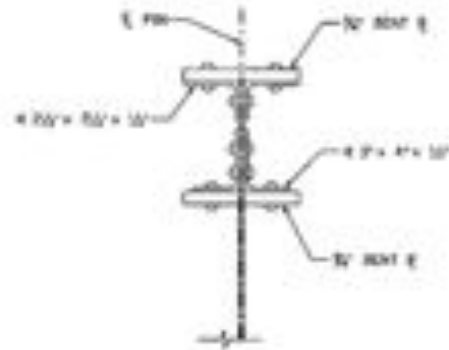
Replacement Portals - 2



DETAIL="I"



SECTION C-C



SECTION 2-0

MOLTELLA

ALL POLYMER COMPOSITIONS ARE GIVEN IN
UNITS MOLES OF CHARGE.

Technical drawing of a bridge section, showing structural details and annotations. The drawing includes various dimensions, material specifications, and construction notes. Key features include:

- Dimensions:** Various measurements are provided, such as 18" x 18" for a hole, 12" x 12" for a hole, and 18" x 18" for a hole. Other dimensions include 12" x 12" for a hole, 12" x 12" for a hole, and 12" x 12" for a hole.
- Material Specifications:** Annotations specify materials like "12" x 12" x 1/2" steel", "12" x 12" x 1/2" steel", and "12" x 12" x 1/2" steel".
- Construction Notes:** Notes describe the construction process, such as "Hole for 12" x 12" x 1/2" steel", "Hole for 12" x 12" x 1/2" steel", and "Hole for 12" x 12" x 1/2" steel".
- Structural Details:** The drawing shows a cross-section of a bridge structure, including a central pier and two side piers. The central pier is labeled "Central Pier" and the side piers are labeled "Side Pier".
- Annotations:** Numerous handwritten notes provide additional information, such as "Hole for 12" x 12" x 1/2" steel", "Hole for 12" x 12" x 1/2" steel", and "Hole for 12" x 12" x 1/2" steel".

The drawing is a detailed technical illustration of a bridge section, showing the structural components and the construction process. The annotations provide valuable information for the construction of the bridge.





The technical drawing consists of four main views of a steel truss bridge deck:

- PLAN:** Shows the top-down view of the deck. It features a central longitudinal section labeled "STRONGER TRUSS". The deck width is divided into sections by vertical stiffeners. Dimensions include overall width of 7'-0" and various segment widths like 2'-0", 6'-0", 2'-0", 2'-0", 2'-0", and 2'-0". A note indicates "SEE TRUSS PLATE DETAIL ON SHEET NO. 1 FOR DETAILS OF STRONGER AND FLOORING TRUSSES".
- ELEVATION:** Shows the side profile of the deck. It details the arrangement of floor beams (labeled "FLOOR BEAM") and their connection to the truss members. Dimensions show a total length of 7'-0" and individual beam spacings of 2'-0", 6'-0", 2'-0", 2'-0", 2'-0", and 2'-0". Notes specify "18 SPS # 2 - 4' x 4' TRUSS" and "18 SPS # 2 - 4' x 4' TRUSS".
- SECTION B-B:** A cross-section view showing the depth of the deck. It illustrates the arrangement of reinforcement bars (rebar) and the concrete slab. Dimensions include a total depth of 1'-0" and specific bar spacings like 4" x 4" x 4" and 18 SPS # 2 - 4' x 4'.
- VIEW A-A:** Another cross-section view, similar to Section B-B, showing the deck's profile and reinforcement details. It includes dimensions for the deck thickness and reinforcement placement.

NOTES:

- SEE "TRUSS FLOOR BEAMS" DETAIL ON SHEET NO. 1
- SEE "REINFORCEMENT" DETAIL ON SHEET NO. 1
- FLOOR BEAMS TO BE FABRICATED WITH 18" DIA. SHIP BOLTS.
- STEEL PLATE AND ANGLES USED TO FABRICATE FLOOR BEAMS SHALL SATISFY MAINTENANCE CODES.
- PAYMENTS FOR FLOOR BEAMS SHALL BE INCLUDED IN ITEM BREAKDOWN "REPAIRS TO BRIDGE STRUCTURE".

② SEE "FLOOR BALANCE" DETAIL ON SHEET 04.
 ③ SEE NOTES FOR FIELD BOLTS.
 FLOOR BEAMS TO BE FABRICATED WITH ④ SEE SHOP DETAILS.
 STEEL PLATE END ANGLE USED TO FABRICATE FLOOR BEAMS
 SHALL MATCH ANCHOR BOLT.
 FURNISH FOR FLOOR BEAMS SHALL BE INCLUDED IN ITEM 04000.
 "REINFORCEMENT ANCHOR BOLT".



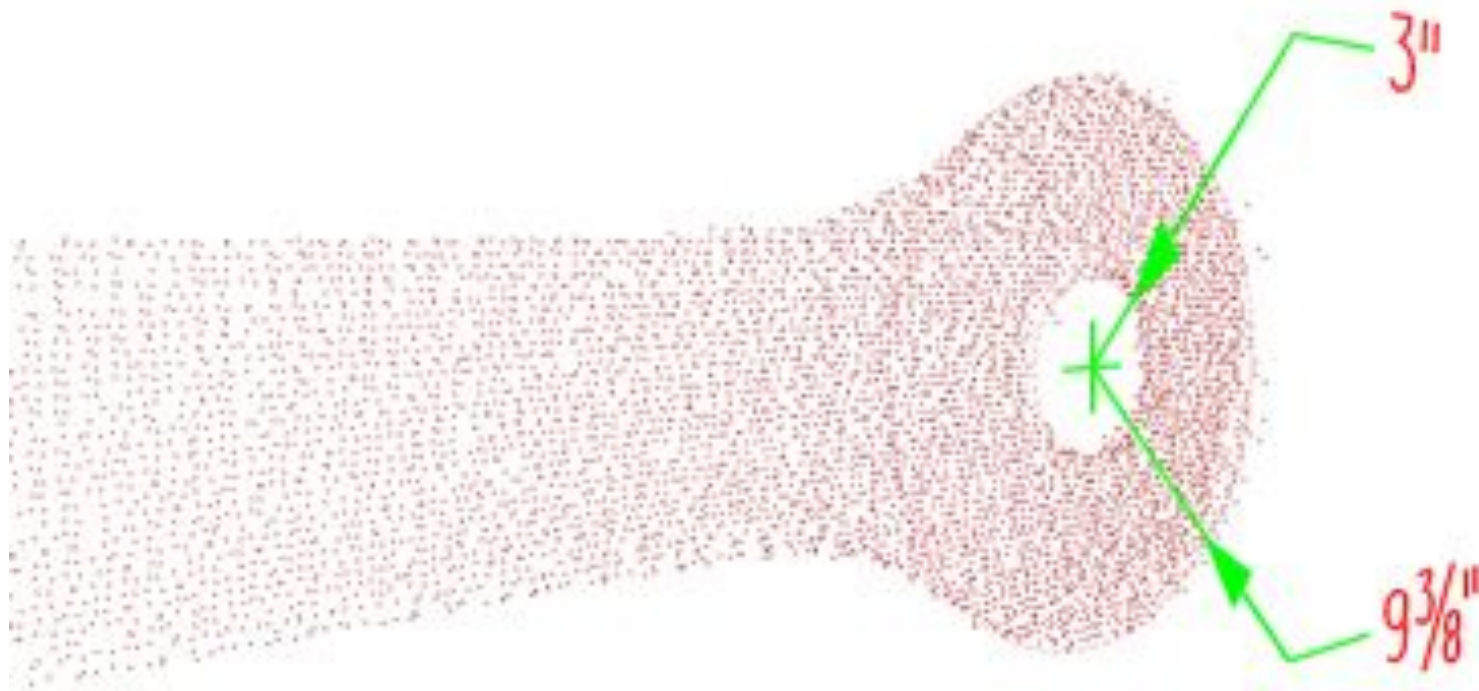


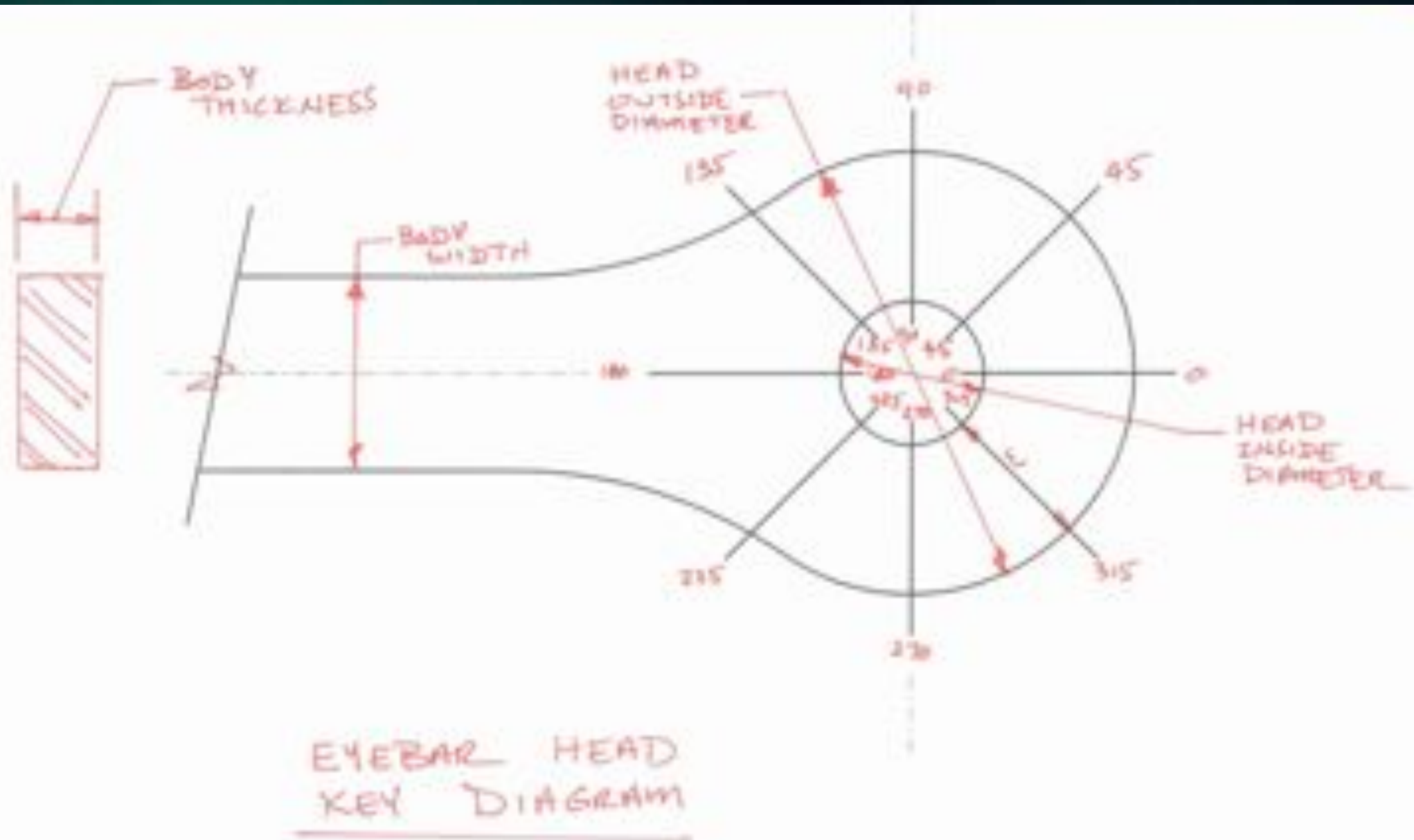
Eyebar Acceptance Criteria

- Started out assuming a stress approach would be used
- Conversation with Dr. Galambos
- Computed some stresses with Timoshenko equations. But what is an acceptable stress?
- Reversed course and ended up using a geometric criteria based on AASHTO/AISC specs
 - Area at the sides needs to be at least 135% of body area
 - Area behind pin needs to be at least 75% of body area



Scan of an eyebar head







90-90 - 1" wide x 1/2" wide gitting x 1/8" deep; 0-0 2" transverse to body x 2" along body x 1/8" deep; diameter at 0-0 is 3-5/16"

REQUIRED AREAS AT DIFFERENT HEAD SECTIONS

Section 0-0	Width	Thickness	Area
Nominal	3.25	1.13	3.66
Full Body Capacity	3.25	1.05	3.41
Minimum Body Capacity	3.25	1.05	3.41
Shop Measured	3.25	1.13	3.58042

Section 45-45	Width	Thickness	Area
Nominal	3.25	1.13	3.66
Full Body Capacity	3.25	1.00	3.25
Minimum Body Capacity	3.25	1.00	3.25
Shop Measured			

Section 90-90	Width	Thickness	Area
Nominal	3.25	1.13	3.66
Full Body Capacity	3.25	0.95	3.08
Minimum Body Capacity	3.25	0.95	3.08
Shop Measured	3.25	1.13	3.59375

Section 135-135	Width	Thickness	Area
Nominal	3.25	1.13	3.66
Full Body Capacity	3.25	1.00	3.25
Minimum Body Capacity	3.25	1.00	3.25
Shop Measured			

Section 180-180	Width	Thickness	Area
Nominal	N/A	1.13	N/A
Full Body Capacity	N/A	1.05	N/A
Minimum Body Capacity	N/A	1.05	N/A
Shop Measured			

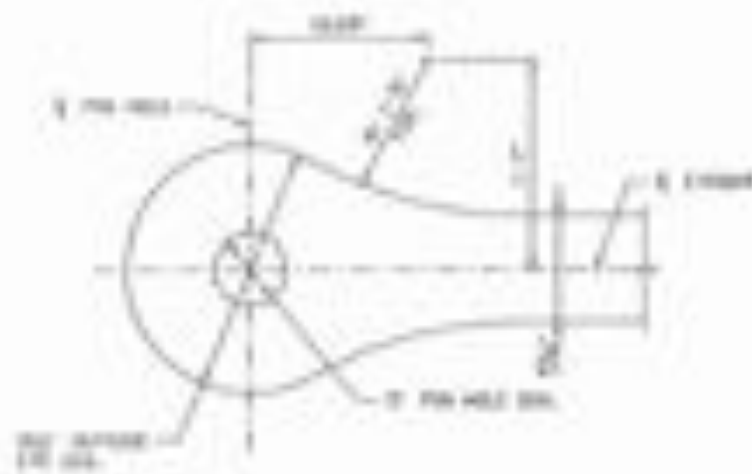
Section 225-225	Width	Thickness	Area
Nominal	3.25	1.13	3.66
Full Body Capacity	3.25	1.00	3.25
Minimum Body Capacity	3.25	1.00	3.25
Shop Measured			

Section 270-270	Width	Thickness	Area
Nominal	3.25	1.13	3.66
Full Body Capacity	3.25	0.95	3.08
Minimum Body Capacity	3.25	0.95	3.08
Shop Measured			

Section 315-315	Width	Thickness	Area
Nominal	3.25	1.13	3.66
Full Body Capacity	3.25	1.00	3.25
Minimum Body Capacity	3.25	1.00	3.25
Shop Measured			

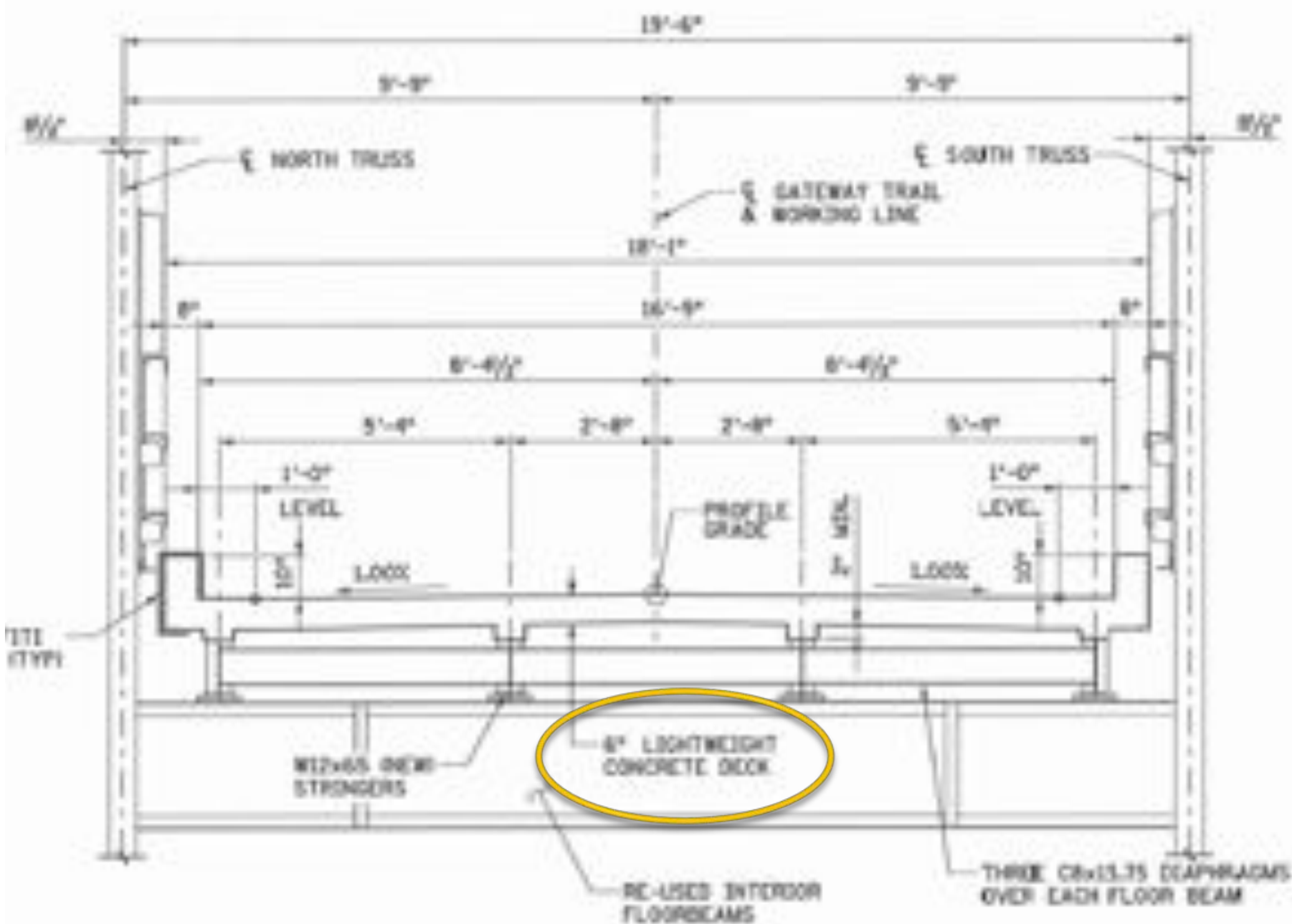


TYPICAL ELEVATION



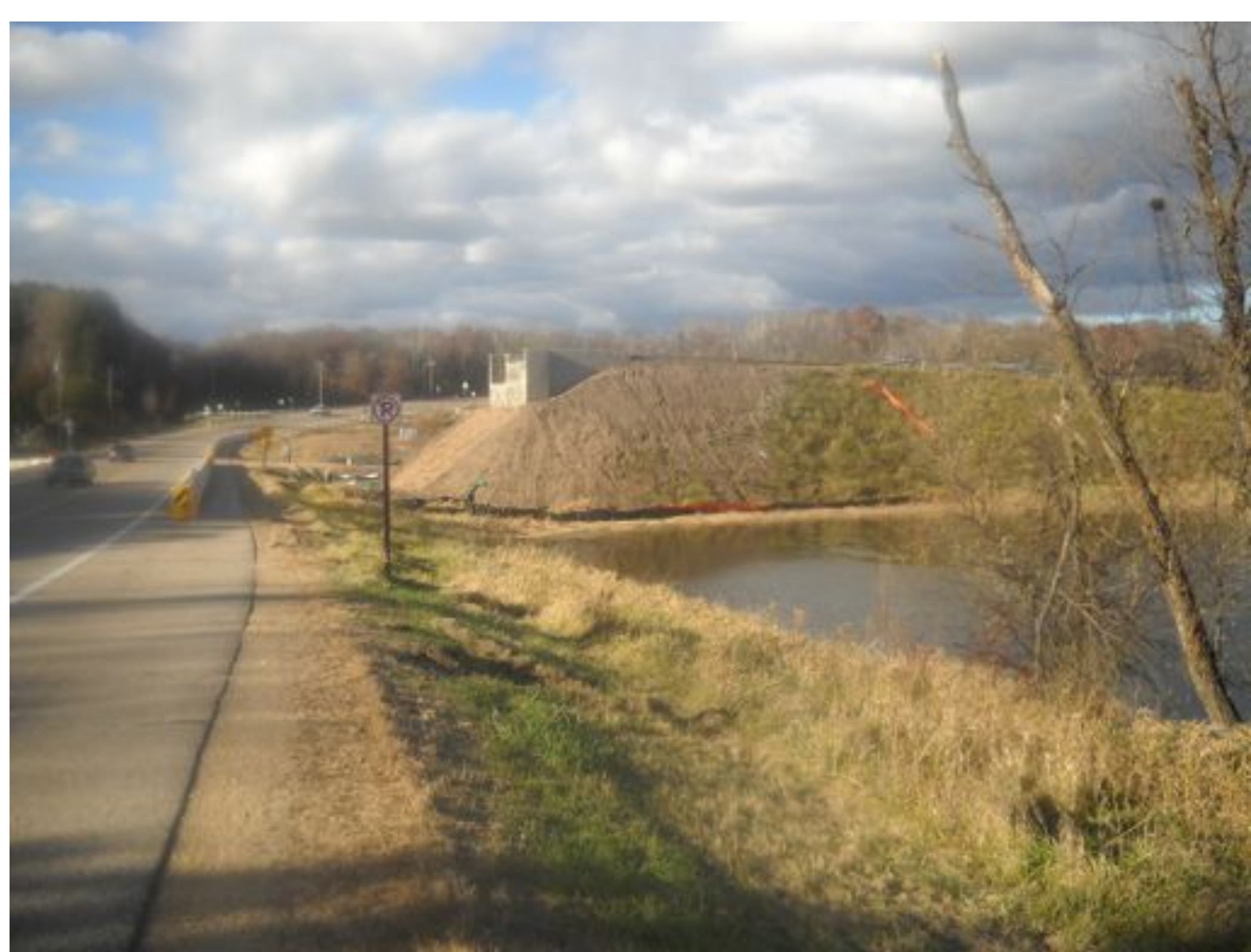
TYPICAL END DETAIL

NEW LOWER CHORD MEMBERS						
CHORD LENGTH	24" DIA. CHORD DIA.	24" DIA. CHORD DIA.	24" DIA. CHORD DIA.	24" DIA. CHORD DIA.	24" DIA. CHORD DIA.	24" DIA. CHORD DIA.
24'-0" TO 26'-0"	24"	24"	24"	24"	24"	24"
26'-0" TO 28'-0"	24"	24"	24"	24"	24"	24"



















Gateway Trail over Manning Ave.

Bridge lift - 05/20/2011





Benefits of Laser-scanning

- Dramatically cut the amount of field time required to collect geometric data for the rehabilitation project. In addition, more information was collected than what we would have planned for up front using older methods.
- Allowed us to readily detail replacement portals
- Allowed us to readily detail replacement floor beams
- Helped us evaluate the eyebars. Eyebars head geometry was pulled from the laser scans.



Rehabilitation of the Phalen Park Arch Bridge (L8560)







2007 Study



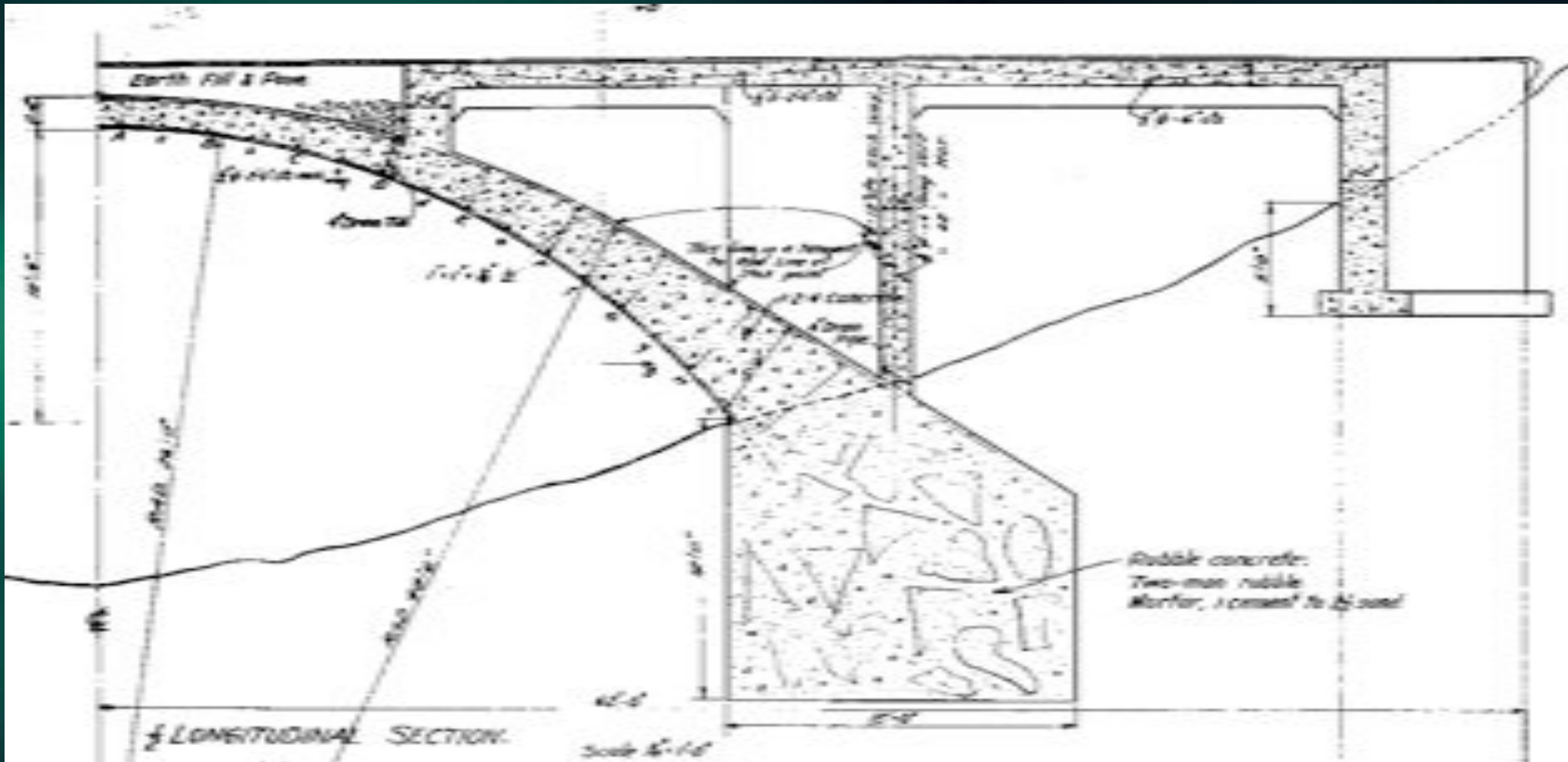


BEAUTIFUL PHALEN PARK, ST. PAUL, MINN.

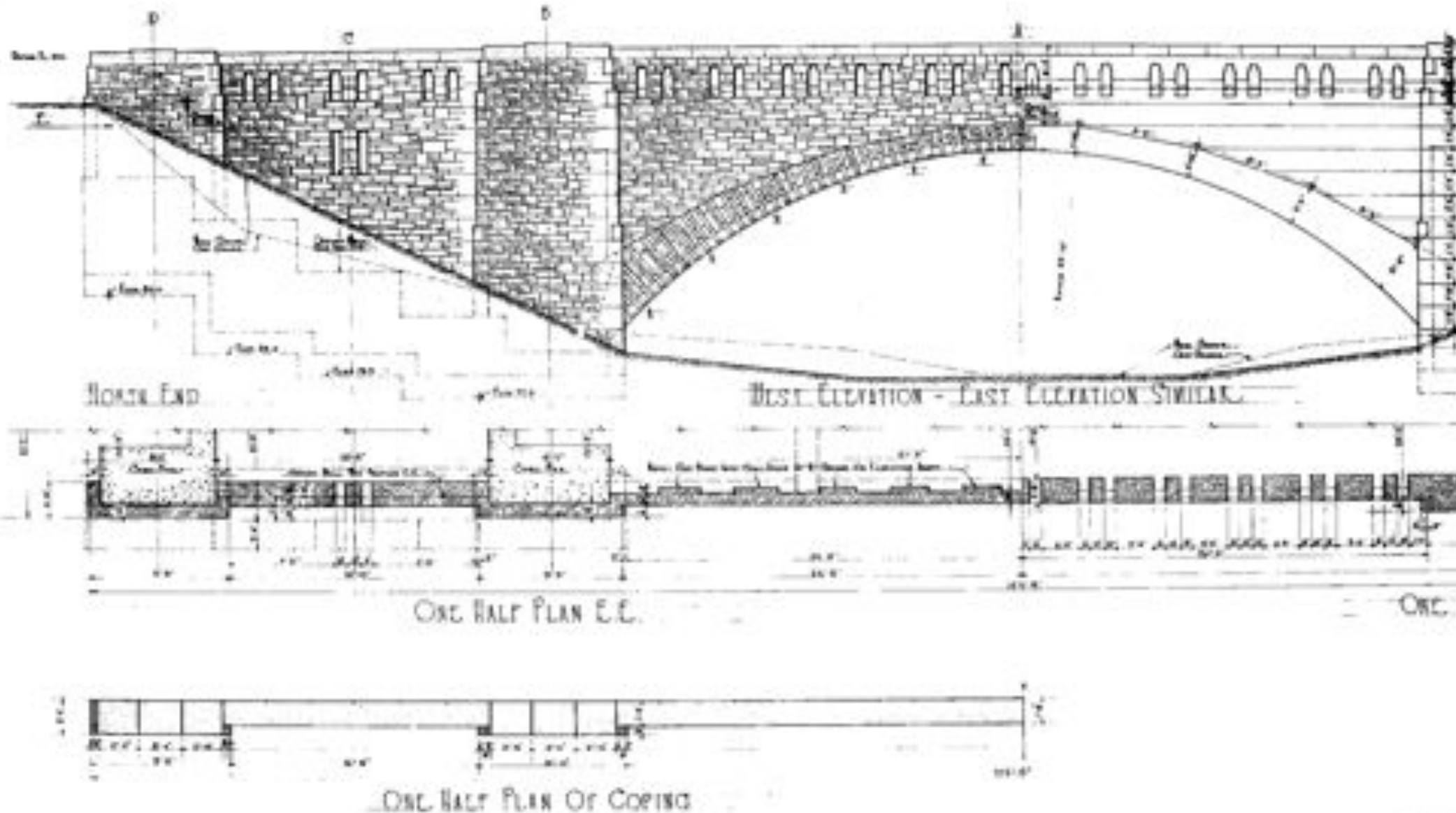
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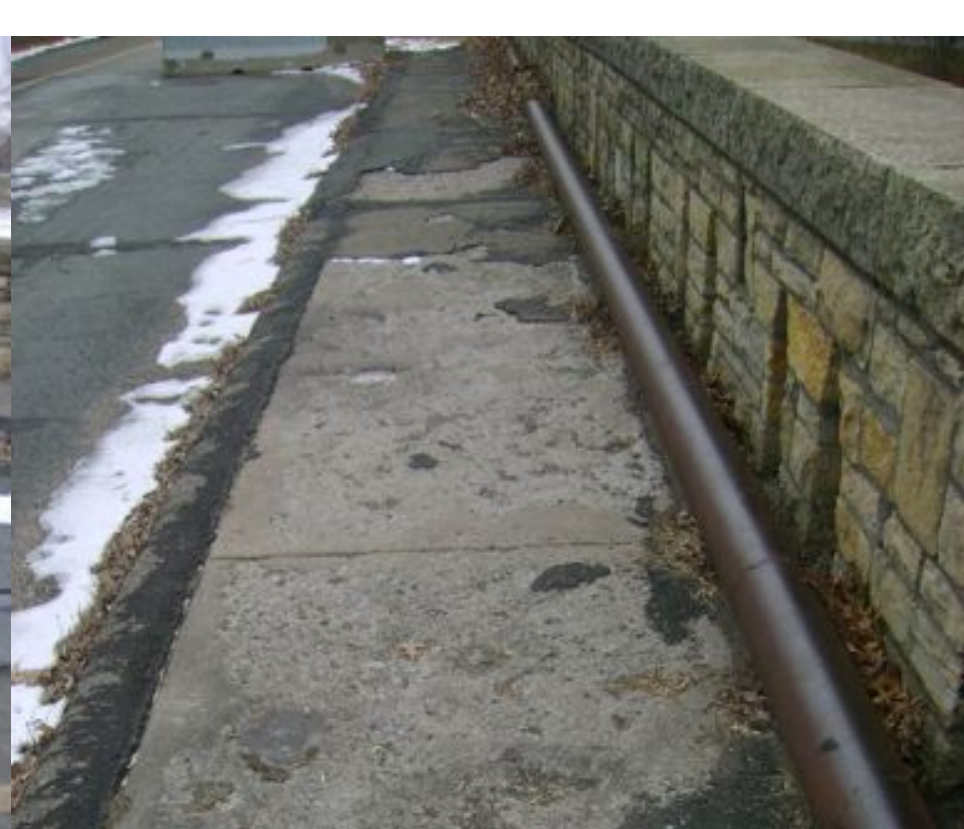


Original 1910 Concrete Arch Details



1934 CWA Rehab





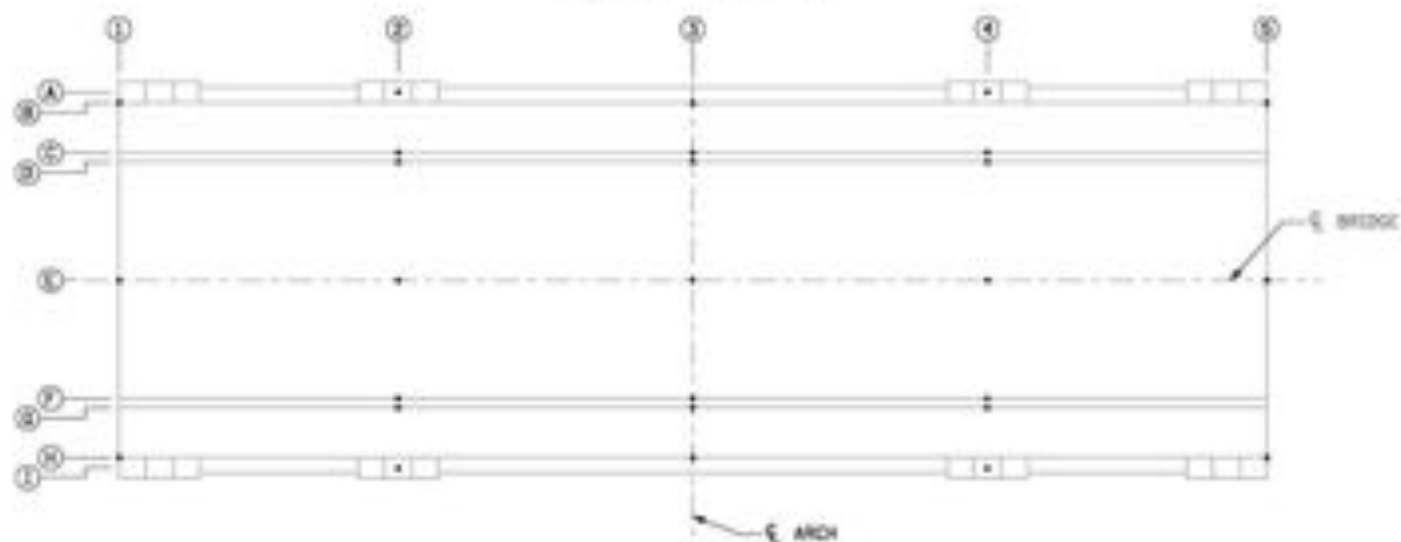
2007 Study





ROW	APPROX. DISTANCE BETWEEN ROWS
A-B	1'-3"
B-C	4'-3"
C-D	1'-0"
D-E	15'-7"
E-F	15'-7"
F-G	1'-0"
G-H	4'-3"
H-I	1'-2"

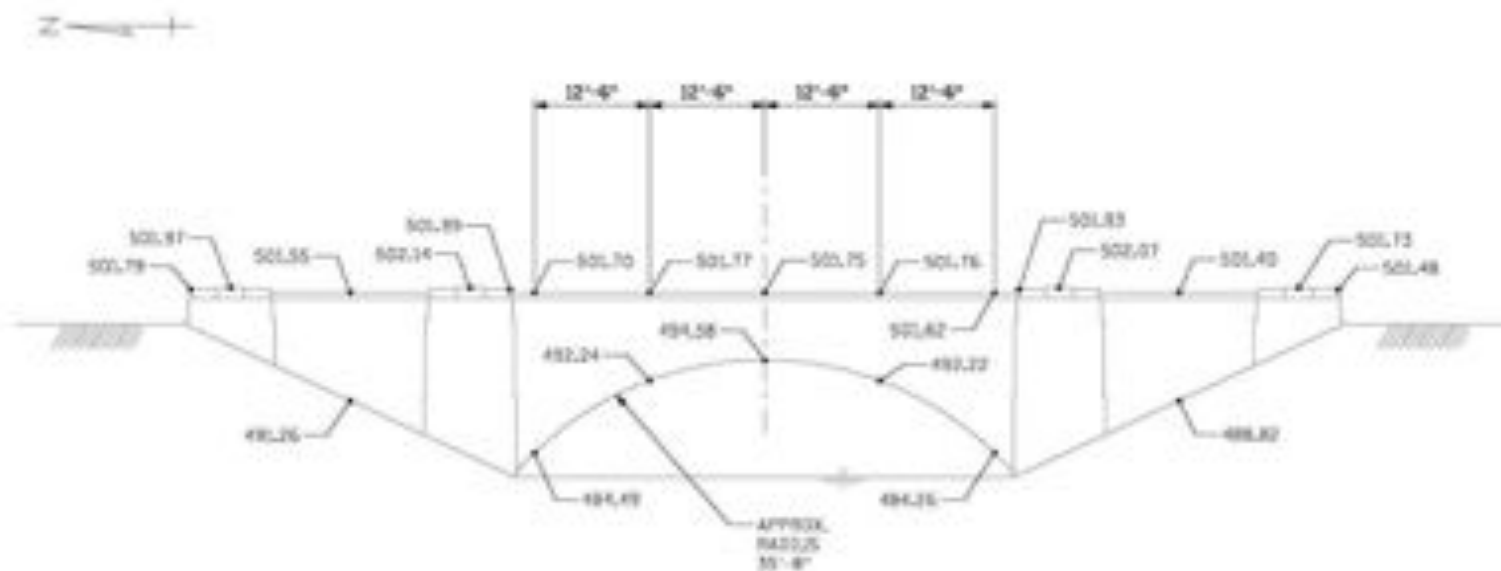
COLUMN	APPROX. DISTANCE BETWEEN COLUMNS
1-2	50'-8"
2-3	31'-1"
3-4	31'-1"
4-5	50'-8"



		ELEVATIONS				
		COLUMN				
ROW	COLUMN	1	2	3	4	5
A		---	502.03	---	502.03	---
B		501.47	---	501.77	---	501.77
C		---	498.48	498.64	498.57	---
D		---	498.00	498.26	498.19	---
E		497.72	498.06	498.21	498.19	497.98
F		---	497.95	498.19	498.13	---
G		---	498.40	498.64	498.49	---
H		501.65	---	501.65	---	501.81
I		---	502.04	---	502.05	---

PHALEN PARK
ARCH BRIDGE
SURVEY
FIGURE 1
1-19-07

HNTB



WEST ELEVATION

PHALEN PARK
 ARCH BRIDGE
 SURVEY
 FIGURE 2
 1-19-07



Compressive Strength Test Results

Sample Number	Core Number	Compressive Strength (PSI)
1	5- to 4-inch-diameter core	7,430
2	7- to 3-inch-diameter core	4,290

Note: core number 6 was not testable due to the poor condition of the core

Results

Chloride Ion Content

Sample Number	Core Number/Location	Sample Depth	Chloride Ion Content (PPM)
1	1 – topside, gutter	0-1 inches	640
2	1	1-2 inches	240
3	2 – topside, roadway	0-1 inches	130
4	2	1-2 inches	130
5	3 – topside, gutter	0-1 inches	91
6	3	1-2 inches	94
7	4 – topside, sidewalk	0-1 inches	510
8	4	1-2 inches	400
9	7 – underside, arch	0-1 inches *	470
10	7	1-2 inches *	860

*Core number 7 was taken from the underside of the concrete arch. Sample depths were referenced to the underside not the topside.

Rehabilitation Strategy

The proposed rehabilitation strategy is based on the following assumptions:

- 1) The arch concrete itself is too delaminated and contaminated with chlorides to be considered a candidate for rehabilitation. Any rehabilitation process is likely to meet a similar fate to that of the earlier applied shotcrete, a fix with a modest service life and future headaches.
- 2) To prevent the continued intrusion of water into the concrete components of the bridge, it is imperative that a new concrete deck system be used to seal the top of the bridge.
- 3) The existing arch structure has sufficient strength to facilitate the construction of the arch liner system



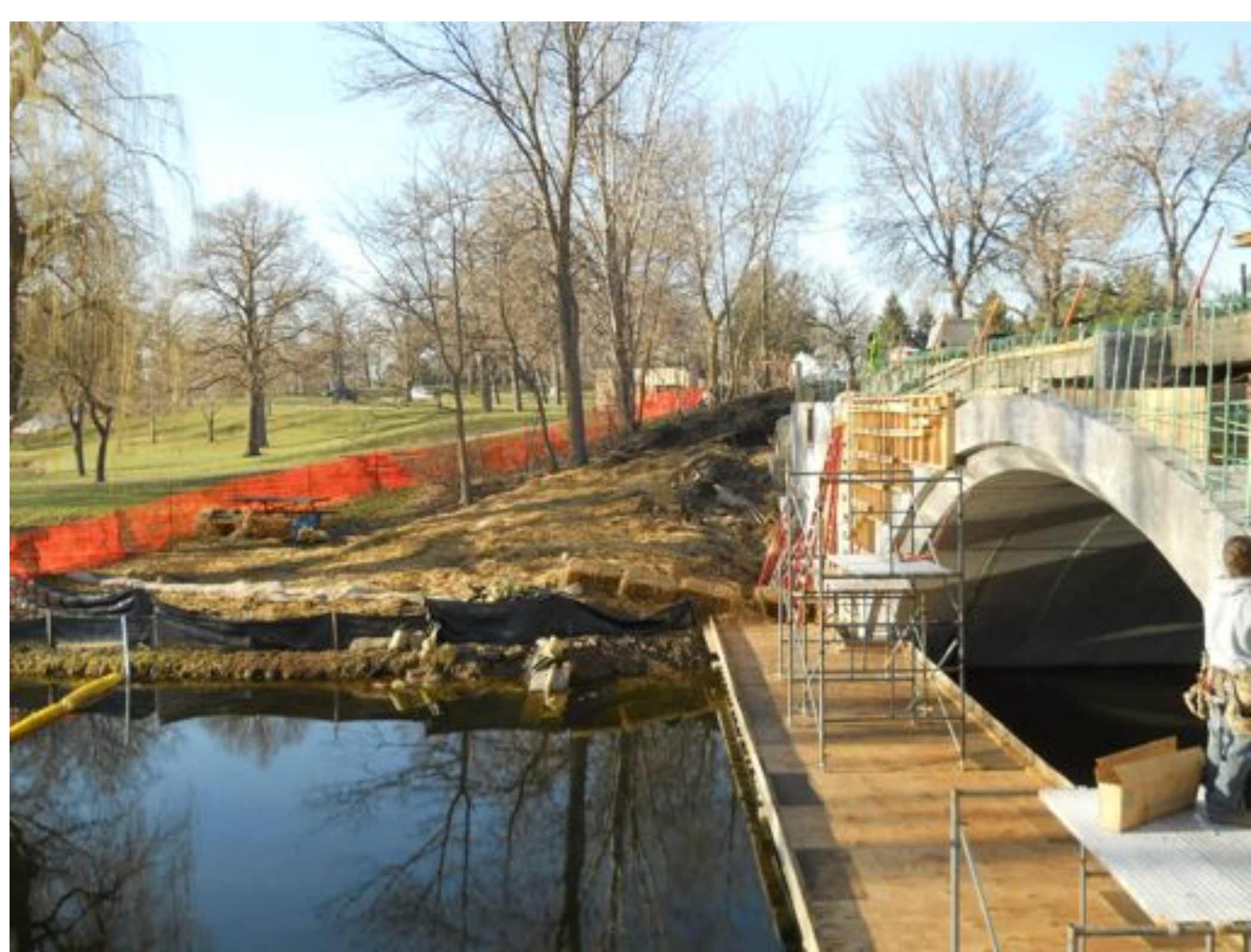




STONE ARCH BRIDGE REHABILITATION

LAKE PHALEN PARK

ST. PAUL, MN



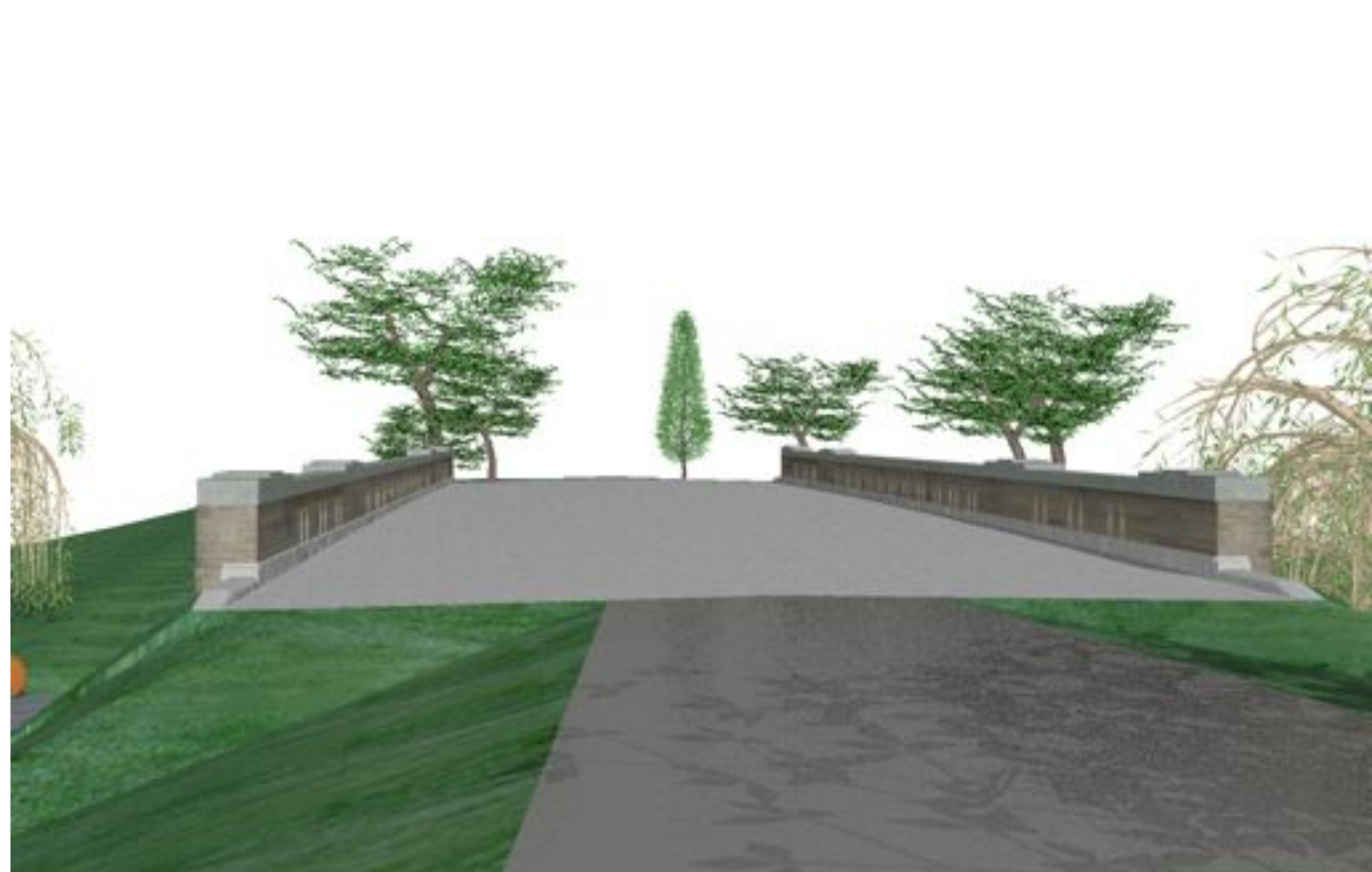














Rehabilitating the 15th and 16th Avenue Bridges over the Midtown Greenway (1916 Reinforced Concrete T-beam bridges)

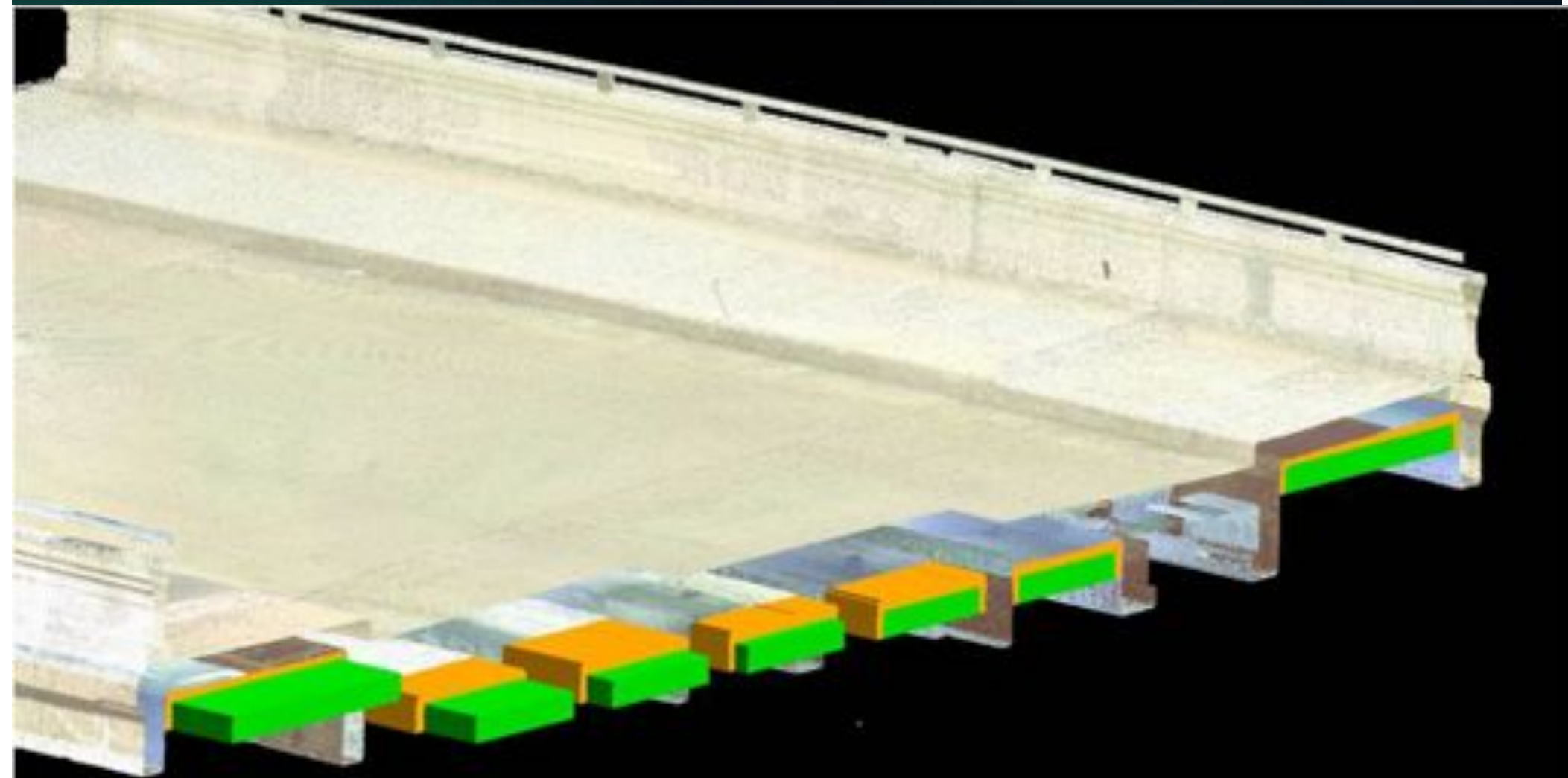




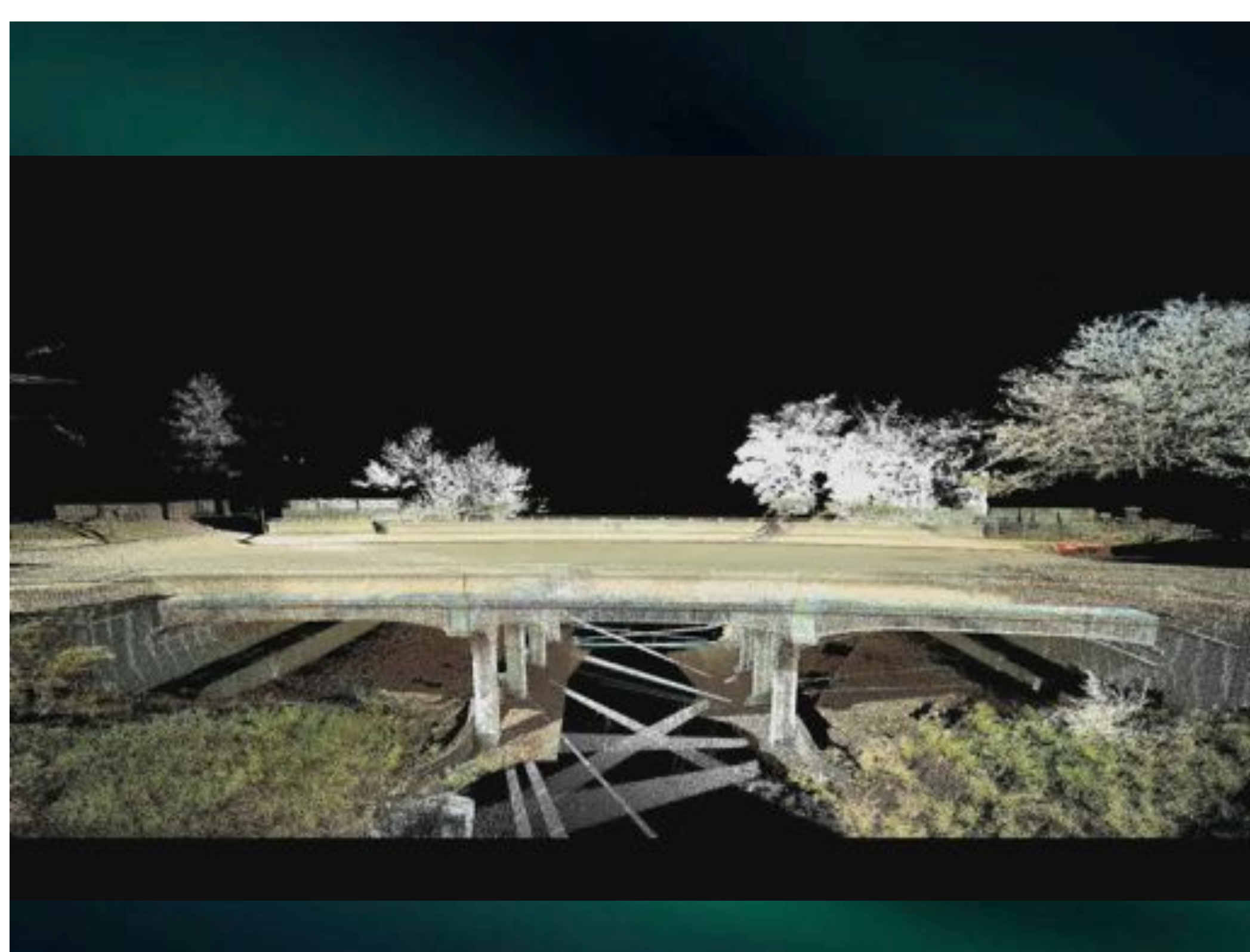
Midtown Greenway Historic District







- SELF-CONSOLIDATING CONCRETE
- PRECAST CONCRETE PANEL



In summary

- For many structures, laser-scanning may prove to be a very cost-effective method to collect field data
- Consider using new materials such as lightweight concrete and self-consolidating concrete
- Be creative and consider providing alternate load paths

